



George Municipality

Traffic Impact Assessment for the Proposed George Campus

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APPROVAL

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1. DEVELOPMENT PARTICULARS

SMEC South Africa (Pty) Ltd was appointed by George Municipality to conduct a Traffic Impact Assessment for the proposed George Campus Development. The site is bound by the Garden Route Dam to the north and Madiba Drive to the south. Refer to Figure 1.



Figure 1 Locality Plan (source: Google)

The site measures approximately 118 hectares in extent. The anticipated composition of the development is a Campus catering for 8 000 students, a Waterfront commercial development of 129 300 square metres Gross Lettable Area (GLA), and a Hotel of 34 500 square metres GLA (assumed to be 345 rooms). The Campus component will include residential units for 303 lecturers and 3 009 students.

For the purpose of this TIA it was assumed that the development will be 50% implemented over 5 years by 2024, and 100% within 10 years by 2029.

2. STUDY AREA

The study area is an area from which transportation elements are selected for the TIA. Such transport elements are selected as follows:

- Site accesses;
- Minimum of two intersections on the road where access is proposed; and
- All roads in sensitive areas.

Taking the above into consideration, the following primary study area and associated transportation elements have been selected for assessment (Refer to Figure 2):

- Stander Street & Site Access 1 (opposite Arthur Bleksley Street);
- Saasveld Road (West) & Site Access 2; and
- Saasveld Road & Site Access 3, opposite Road 1.

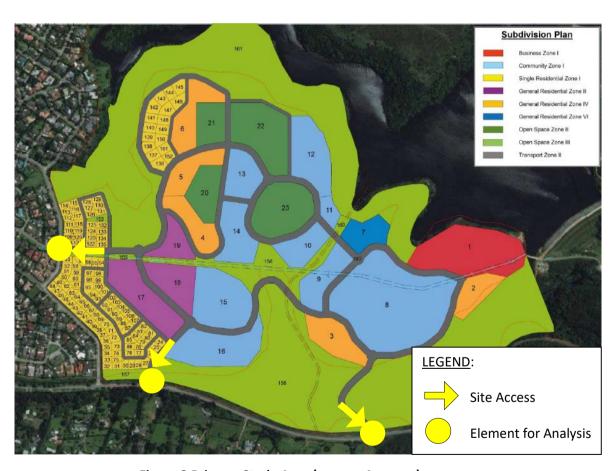


Figure 2 Primary Study Area (source: Aurecon)

Based on the type and extent of development, the following secondary study area and associated transportation elements have been selected for assessment (Refer to Figure 3):

- N9 Knysna Street & Saasveld Road intersection;
- N9 Knysna Street & Road 1; and
- Saasveld Road & Meyer Street.

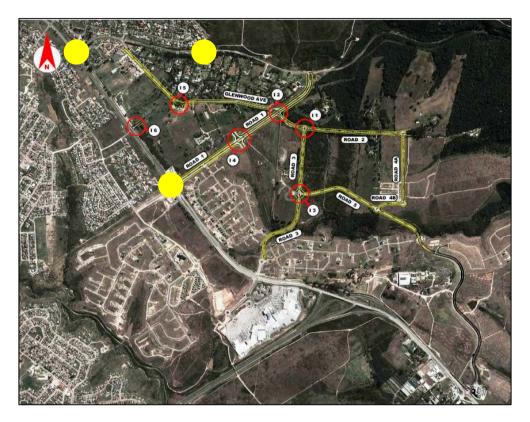


Figure 3 Secondary Study Area (source: Google)

3. BACKGROUND INFORMATION

3.1 Existing Roads

National Route N9 is a Class 2 Major Arterial under the jurisdiction of the South African National Road Agency Limited. In the vicinity of Saasveld Road it comprises of two lanes per direction. It experiences moderate traffic flows during peak hours, and operates at an acceptable Level of Service.

Saasveld Road is a Class 3 Minor Arterial, extending from Eden George to the north of Wilderness and Hoekwil. The road comprises of one lane per direction in the vicinity of the subject site. It experiences low traffic flows during peak hours, and operates at an acceptable Level of Service.

Meyer Street is a Class 4 Urban Collector, serving the suburb of Eden, George. The road comprises of one lane per direction in the vicinity of the subject site. It experiences low traffic flows during peak hours, and operates at an acceptable Level of Service.

Kraaibosch Way is a Class 4 Urban Collector, designed to predominantly serve the Kraaibosch development. The will comprises of one lane per direction. It experiences low traffic flows during peak hours, and operates at an acceptable Level of Service.

3.2 Public Transport Facilities

George is currently served by three phases of the George Integrated Public Transport Network (George IPTN). As Kraaibosch and George Campus is rolled out, it is anticipated that these developments will be well served by an extended Phase 1 of the George IPTN. Refer to Figure 4.

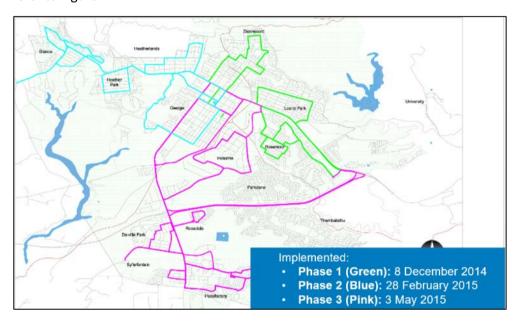


Figure 4 Public Transport Facilities (source: George Municipality)

3.3 Non-Motorized Transport Facilities

The George Campus design focuses on pedestrian accessibility and mobility, providing green corridors linking all components of the development. Refer to Figure 5.

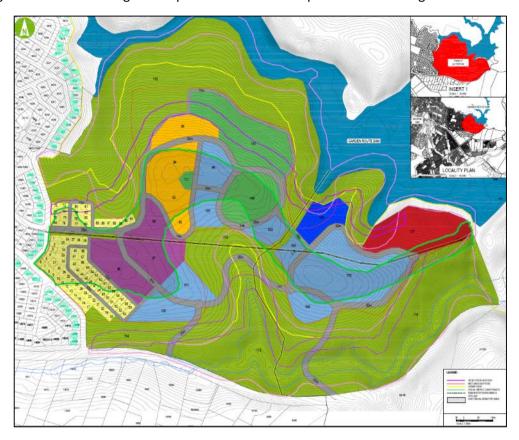


Figure 5 NMT Facilities (source: Aurecon)

3.4 Planned Changes to Transportation Facilities

It is proposed that an extended Phase 1 of the George IPTN serve the George Campus, with the provision of bus stops within the Campus grounds.

3.5 Site Access

The site will be served by three accesses, as follows:

- Access 1 along Stander Street (opposite Arthur Bleksley Street);
- Access 2 along Saasveld Road (between Meyer Street & Access 3); and
- Access 3 along Saasveld Road (opposite Road 1).

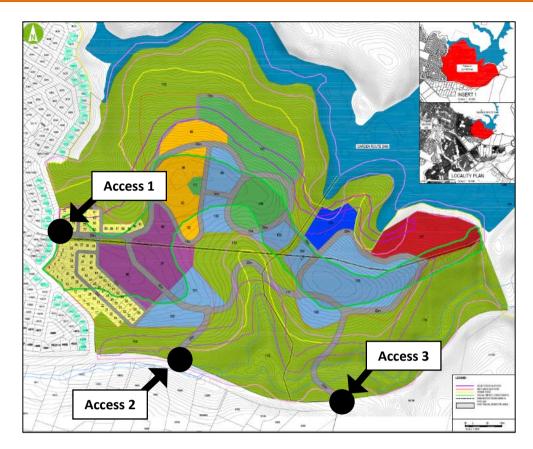


Figure 6 Site Access (source: Aurecon)

The access spacing requirements were derived from the COTO TMH 16 Volume 2. This requires a 600 metre access spacing (± 20%) along Class 3 roads within Urban Areas.

With the locations of Meyer Street and Access 3 being fixed, it would be preferred to locate Access 2 midway between Meyer Street and Access 3. This was, however, not achievable, due to environmental constraints limiting the possible access locations.

Taking into consideration the proposed junction control being roundabouts, it would be deemed appropriate to accept a reduced intersection spacing on either side of Access 2. The attainable access spacing along Saasveld Road is 300 metres between Meyer Street and Access 2, and 600 metres between Access 2 and Access 3.

4. OTHER PLANNING AUTHORITIES

N9 Knysna Street falls under the jurisdiction of the South African National Roads Agency Limited (SANRAL), and Saasveld Road under the Western Cape Department of Transport. As such, these Authorities would need to be included in the approval process.

5. TRAFFIC DEMAND ESTIMATION

5.1 Assessment Year

The traffic assessment will be undertaken for a 2024 and 2029 design year. A linear buildout of the development has been assumed, as set out in Table 1.

Table 1 Development Phasing (Cumulative)

Phase	Year	Assumed Build-Out	University (students)	Housing (units)	Commercial (sqm GLA)	Hotel (rooms)
Phase 1	2024	50%	4 000	1 652	64 650	173
Phase 2	2029	100%	8 000	3 303	129 300	345

5.2 Assessment Hour

The traffic assessment must be undertaken for the hours during which the combined effect of background and development traffic will result in the highest traffic demand. Taking into consideration the planned mixed use development, it is deemed appropriate for the Weekday AM and PM Peak Hours to be analysed.

5.3 Background Traffic Demand Estimation

5.3.1 Traffic Counts

Manual classified intersection traffic counts were undertaken as part of this project assignment. Details of the traffic survey are provided below:

Date counted July 2019

Day of the week
 Normal Weekdays

Day class
 Normal

Congestion levels Low

• Enumerator SMEC

5.4 Peak Hour

A common peak hour was identified for the intersections under discussion, as follows:

Weekday AM Peak Hour 07h00 - 08h00

• Weekday AM Peak Hour 16h15 – 17h15

5.5 Traffic Growth

A traffic growth rate is applied to background traffic in order to determine the anticipated increase in Base Year traffic by a predefined Design Year.

The COTO TMH 17 South African Trip Data Manual dated September 2012 provides typical growth rates to be used for growth areas based on the existing/anticipated rate of growth. Refer to Table 2.

Table 2 Typical Growth Rates

DEVELOPMENT AREA	GROWTH RATE
Low Growth Areas	0% - 3%
Average Growth Areas	3% - 4%
Above Average Growth Areas	4% - 6%
Fast Growing Ares	6% - 8%
Exceptionally High Growth Areas	> 8%

Taking into consideration the location of the subject site, a compounded traffic growth rate of 2.0% was applied to the 2019 Base Year Traffic in order to derive 2024 and 2029 Design Year traffic flows.

Taking into consideration the close proximity of the other development parcels forming part of the Kraaibosch development, it was deemed appropriate to only apply a growth rate to N9 Knysna Street traffic.

5.6 Existing exercised land-use rights

Where a development has existing land-use rights that have been exercised and where a growth rate is applied, the trip generation of the exercised rights must be estimated and subtracted from the traffic counts before any growth is applied.

No existing exercised land-use rights apply to this development.

5.7 Trip Generation by Other Developments

Other developments as well as future potential development in the area must be taken into account in the estimation of future background traffic. The following developments have been taken into account:

5.7.1 Kraaibosch Development

The Roads Master Plan for the Kraaibosch Development dated September 2018 includes several land parcels and development land therein.

The location of each development is shown in Figure 7.

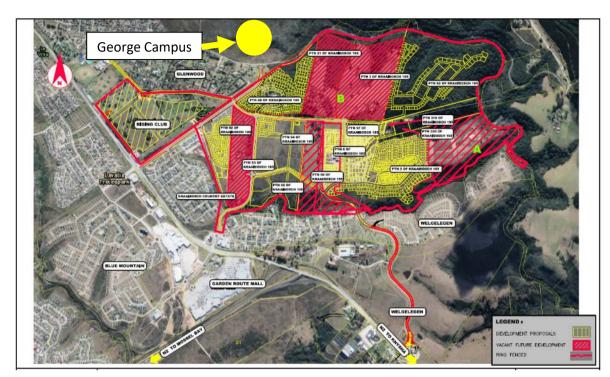


Figure 7 Other Developments (source SMEC)

The trip generation potential of the other developments is set out in Table 3.

Table 3 Other Development Trip Generation

PORTION	PROPOSED DEVEL ORMENT	TOID DATE			TRIPS		TOTAL
PORTION	PROPOSED DEVELOPMENT	IRIP KATE					TOTAL
	43 High Income Units	15				110	146
195/62	5.2 ha Group Housing Units @ 55 du/ha	TRIP RATE	234	312			
195/3	30.5 ha Retirement Units @ 20du/ha					69	92
	144 Group Housing Units	1.1	25%	75%	40	119	159
195/88	124 High Income Units	1.5	25%	75%	46	140	186
195/21	20 ha High Income Units @ 15du/ha	1.5	25%	75%	112	338	450
	8.76 ha Retirement @ 35 du/ha	0.15	35%	65%	16	30	46
	2.27 ha Group Housing units @ 55 du/ha	1.1	25%	75%	35	104	138
	1.60 ha Flats @ 55du/ha	1.1	25%	75%	24	73	97
195/54 & 55	2.76 ha Suburban Medical Centre	7/100m ²	55%	45%	1061	868	1929
	6.05 ha Private Hospital (50% coverage)	2.4/100m ²	55%	45%	399	327	726
	1.54 ha Shopping Centre	224.5 GLA-0.34 / 100m ²	50%	50%	653	653	1306
	171 Retirement Units	0.15	35%	65%	9	17	26
195/52	13 Retirement Units	0.15	35%	65%	1	1	2
	1.43 ha Group Housing Units @ 15 du/ha	1.1	25%	75%	6	18	24
	0.36 ha Sport/Recreation	40/ha	50%	50%	7	7	14
	156 Group Housing Units	1.1	25%	75%	43	129	172
405/5	0.75 ha Community Orientated Uses	40/ha	50%	50%	15	15	30
195/5	289 Retirement Units	0.15	25%	75%	11	32	43
	40 High Income Units	1.5	25%	75%	15	45	60
195/6 & 57	343 Retirement Units	0.15	25%	75%	13	39	52
195/1	124 Group Housing units	1.1	25%	75%	34	102	136
195/56	14.00 ha Vacant land @ 15 du/ha	1.1	25%	75%	58	173	231
195/53	13.60 ha Vacant land @ 15du/ha	1.1	25%	75%	56	168	224
	6.04 ha Sport/Recreation/Education	40/ha	50%	50%	121	121	242
Riding Club	7.75 ha Community Orientated Uses	40/ha	50%	50%	155	155	310
	9.86 ha Group Housing @ 15 du/ha	1.1	25%	75%	41	122	163
195/319	5 High Income units	1.5	25%	75%	2	6	8
	0.74 ha Group Housing @ 60du/ha	1.1	25%	75%	12	37	49
405/200	5 High Income units	1.5	25%	75%	2	6	8
195/320	0.75 ha Group Housing @ 60du/ha	1.1	25%	75%	13	38	50
Section A	18.60 ha Vacant land @ 15du/ha	1.1	25%	75%	77	230	307
						TOTAL	7738

The anticipated trip generation for the other developments totals to 7 738 private vehicle trips during the Weekday AM Peak Hour.

With reference to the Kraaibosch Roads Master Plan and Cost Apportionment (Revision 4) dated September 2018, it is not feasible to analyse the operational analysis of the infrastructure until the site development plans have reached a certain level of finality. As such, this development impact is not considered as part of the current project assignment.

5.8 Trip Generation

Trip generation rates are measured in units of trip ends, with either an origin or a destination at the development. It is the sum of traffic to or from a development.

The Trip Generation Rates for the planned land use types were obtained from the COTO TMH 17 South African Trip Data Manual dated September 2012.

The trip generation potential of Phase 1 of the George Campus is shown in Table 4.

Table 4 Trip Generation - Phase 1

		Trip Generation Unit Rate		Total Trips				
Land Use	Unit			AM		PM		
		AM	PM	In	Out	In	Out	
Hotel (rooms)	173	0.5	0.5	52	35	48	39	
University (students)	4 000	0.2	0.2	640	160	240	560	
Shopping Centre (sqm GLA)	64 650	0.6	3.0	330	178	1438	1438	
Total		•		1 022	372	1 725	2 037	
	13	94	3 7	62				

Based on the size of the Phase 1 retail component, a site-specific size adjustment factor of 1.308 applies.

The trip generation potential of Phase 2 of the George Campus is shown in Table 5.

Table 5 Trip Generation - Phase 2

		Trip Generation Unit Rate		Total Trips			
Land Use	Unit			AM		PM	
		AM	PM	In	Out	In	Out
Hotel (rooms)	345	0.5	0.5	104	69	95	78
University (students)	8 000	0.2	0.2	1 280	320	480	1 120
Shopping Centre (sqm GLA)	129 300	0.6	3.0	584	314	2 546	2 546
Total				1 968	703	3 121	3 743
	2 6	71	6 80	64			

Based on the size of the Phase 2 retail component, a site-specific size adjustment factor of 1.158 applies.

It is anticipated that Phase 1 of the planned development would generate 1 394 and 3 762 new vehicular trips during the Weekday AM and PM Peak Hours respectively, and with Phase 2 it would generate a total of 2 671 and 6 864 new vehicular trips during the Weekday AM and PM Peak Hours respectively.

5.9 Trip Reduction Factors

For the purpose of this study, the below trip reduction factors from the George Campus were applied, subject to approval by George Municipality. Particular note should be taken of the Retail component, which is specifically designed for the needs of the Campus. As such, it was deemed appropriate to assess this component of the development serving very low car ownership. Trip reduction factors for transit and mixed use were applied to the remainder of the development components. Refer to Table 6.

Table 6 Trip Reduction Factor

		Adjustment						
Land Use	Mixed	Car Ow	nership	Transit	Costor			
	Use	Low	Very Low	Corridors	Factor			
University	20%			15%	0.68			
Hotel, Residential	20%			15%	0.68			
Shopping Centre	10%		60%	15%	0.31			

Taking into consideration the trip reduction factors being applied, the revised vehicular trip generation potential for Phase 1 is shown in Table 7.

Table 7 Revised Vehicular Trip Generation - Phase 1

		Trip Total Trips Generation					
Land Use	Unit	Rate		AM		PM	
		AM	PM	In	Out	In	Out
Hotel (rooms)	173	0.5	0.5	35	24	32	26
University (students)	4 000	0.2	0.2	435	109	163	381
Shopping Centre (sqm GLA)	64 650	0.6	3.0	101	54	440	440
Total				571	187	635	847
				75	58	1 48	33

Similarly, the revised vehicular trip generation potential for Phase 2 is shown in Table 8.

Table 8 Revised Vehicular Trip Generation - Phase 2

		Trip Total Trips Generation					
Land Use	Unit	Rate		AM		PM	
		AM	PM	In	Out	In	Out
Hotel (rooms)	345	0.5	0.5	70	47	65	53
University (students)	8 000	0.2	0.2	870	218	326	762
Shopping Centre (sqm GLA)	129 300	0.6	3.0	179	96	779	779
Total				1 119	361	1 170	1 593
				1 4	-80	2 70	63

It is anticipated that Phase 1 of the planned development would generate 758 and 1 483 new vehicular trips during the Weekday AM and PM Peak Hours respectively, and with Phase 2 it would generate a total of 1 480 and 2 763 new vehicular trips during the Weekday AM and PM Peak Hours respectively.

5.10 Trip Types

For the purpose of this study, it is assumed that all trips associated with the proposed development are classified as primary trips, therefore new trips on the surrounding road network.

6. TRIP DISTRIBUTION AND ASSIGNMENT

6.1 Trip Distribution – Internal

The location and extent of individual land use parcels within the development will define the access to be used in serving those components. With this in mind, the anticipated internal trip distribution is shown in Table 9.

Table 9 Internal Trip Distribution

Component	Access 1	Access 2	Access 3	
University	40%	30%	30%	
Hotel		100%		
Retail	10%	40%	50%	

6.2 Trip Distribution – External

Trip distribution was estimated manually, based on the principles of the gravity model and with knowledge of local conditions. Refer to Table 10.

Table 10 External Trip Distribution

Direction	Destination	Route	Distribution
SW	George CBD	N9 Knysna Street	40%
W	George CBD	Stander Street	10%
W	George Bodorp	Stander Street	20%
S	Rosemore	Kraaibosch Way	20%
E	N2	N9 Knysna Street	10%

Based on the trip generation potential of the subject site, development trip distribution summary is set out in Table 11.

Table 11 Development Trip Distribution

Direction	Route	Percent	AM In	AM Out	PM In	PM Out
SW	N9 Knysna Street	40%	448	144	468	637
W	Stander Street	30%	336	108	351	478
S	Kraaibosch Way	20%	224	72	234	319
Е	N9 Knysna Street	10%	112	36	117	159
Total		100%	1119	361	1170	1593

6.3 Traffic Assignment

Traffic assignment involves determining the percentage of traffic that will use specific routes in the network. The traffic assignment is made with consideration to logical routings, available roadway capacity, right-turn movements, travel times and other factors. Refer to Figure 8.

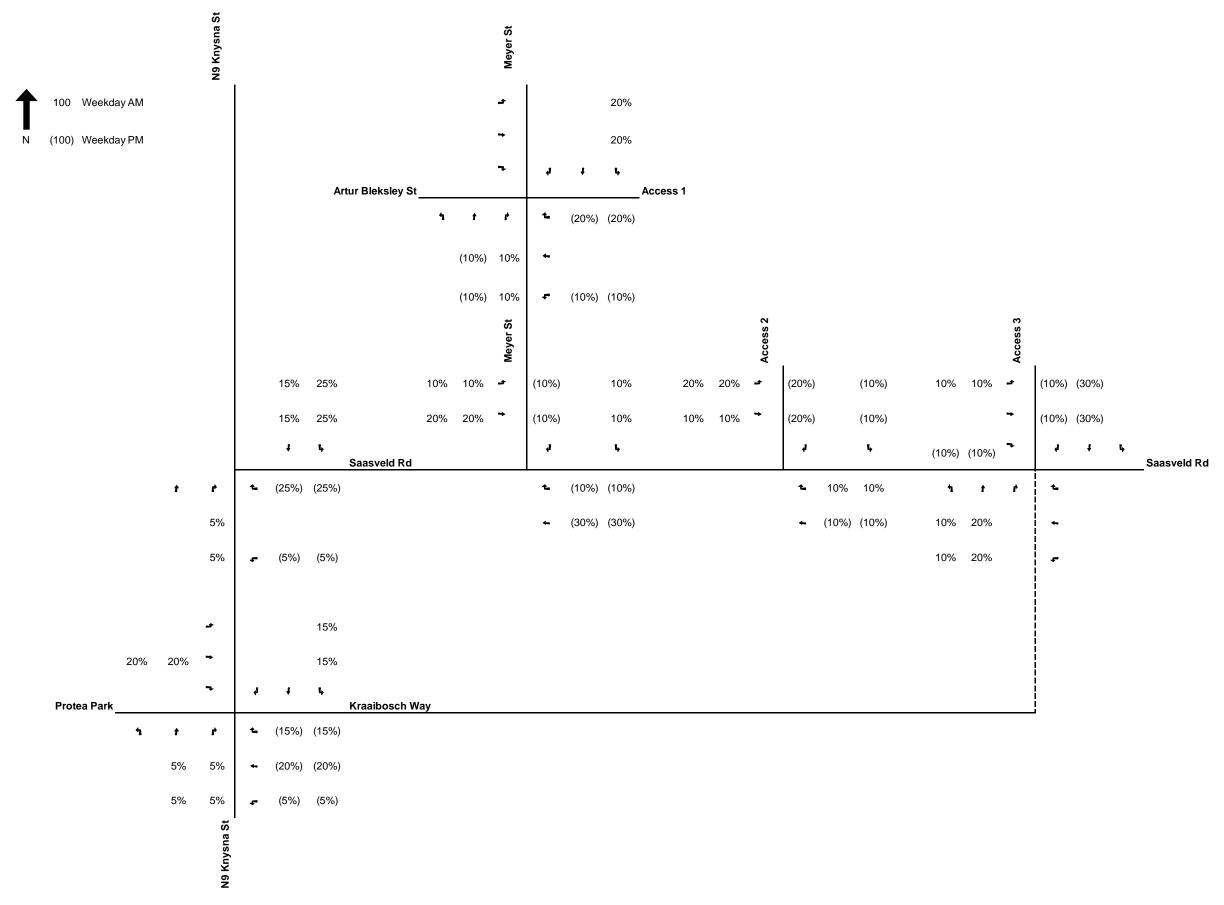


Figure 8 Traffic Assignment

7. TOTAL TRAFFIC DEMAND

7.1 Figures

The following information on traffic demand is provided for each horizon year and peak hour that is assessed:

- Figure 9 2019 Base Year Traffic;
- Figure 10 Phase 1 Development Trips;
- Figure 11 Phase 1+2 Development Trips;
- Figure 12 2024 Design Year + Phase 1 Development Trips: and
- Figure 13 2029 Planning Year + Phase 1+2 Development Trips.

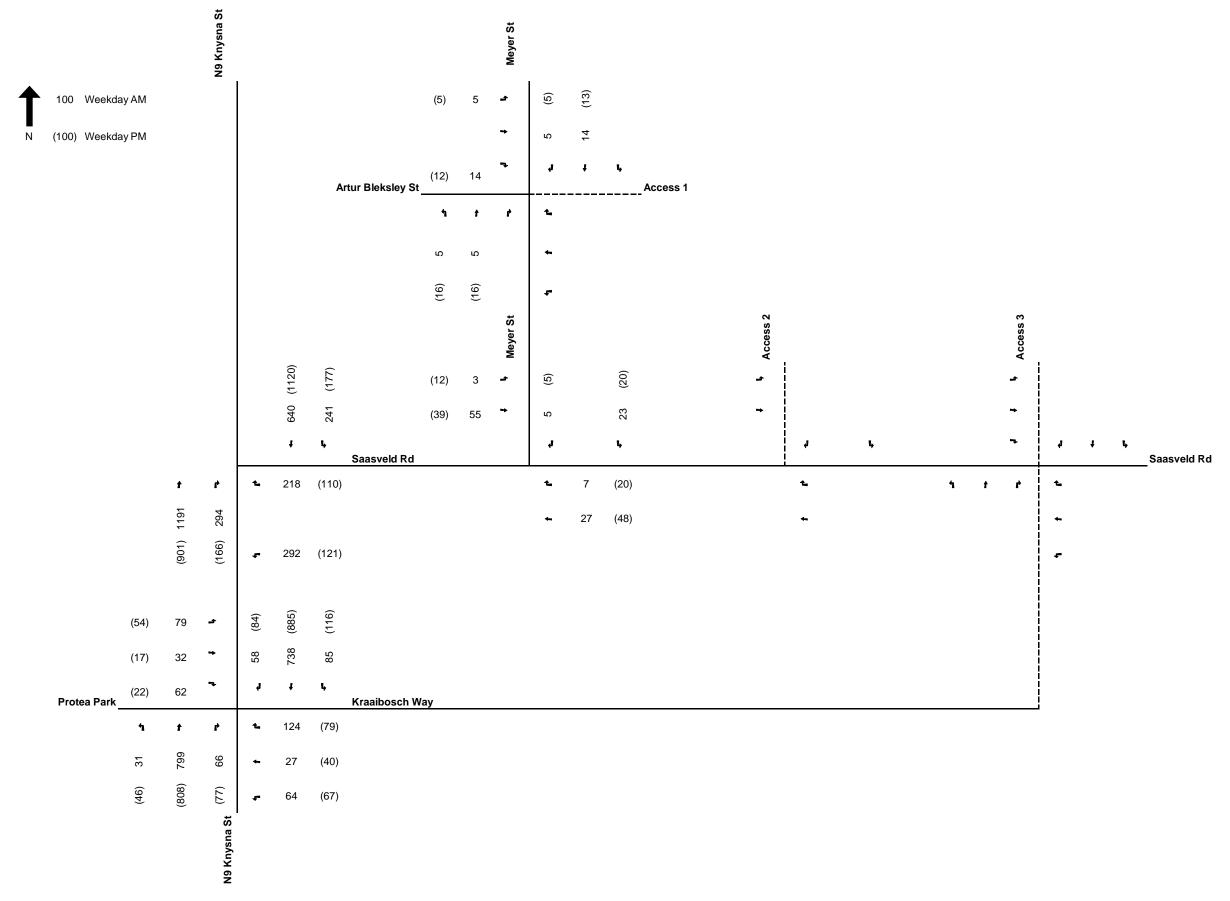


Figure 9 2018 Base Year Traffic

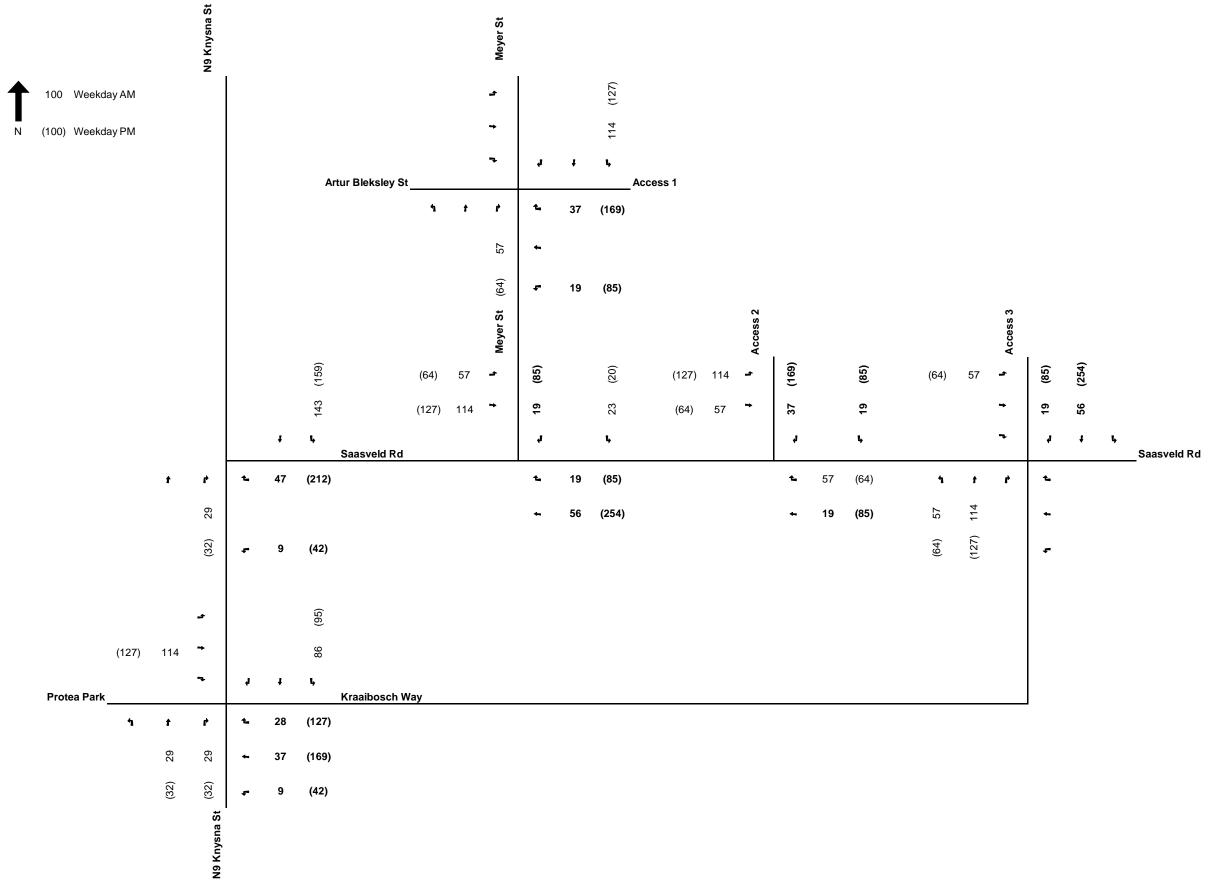


Figure 10 Phase 1 Development Trips

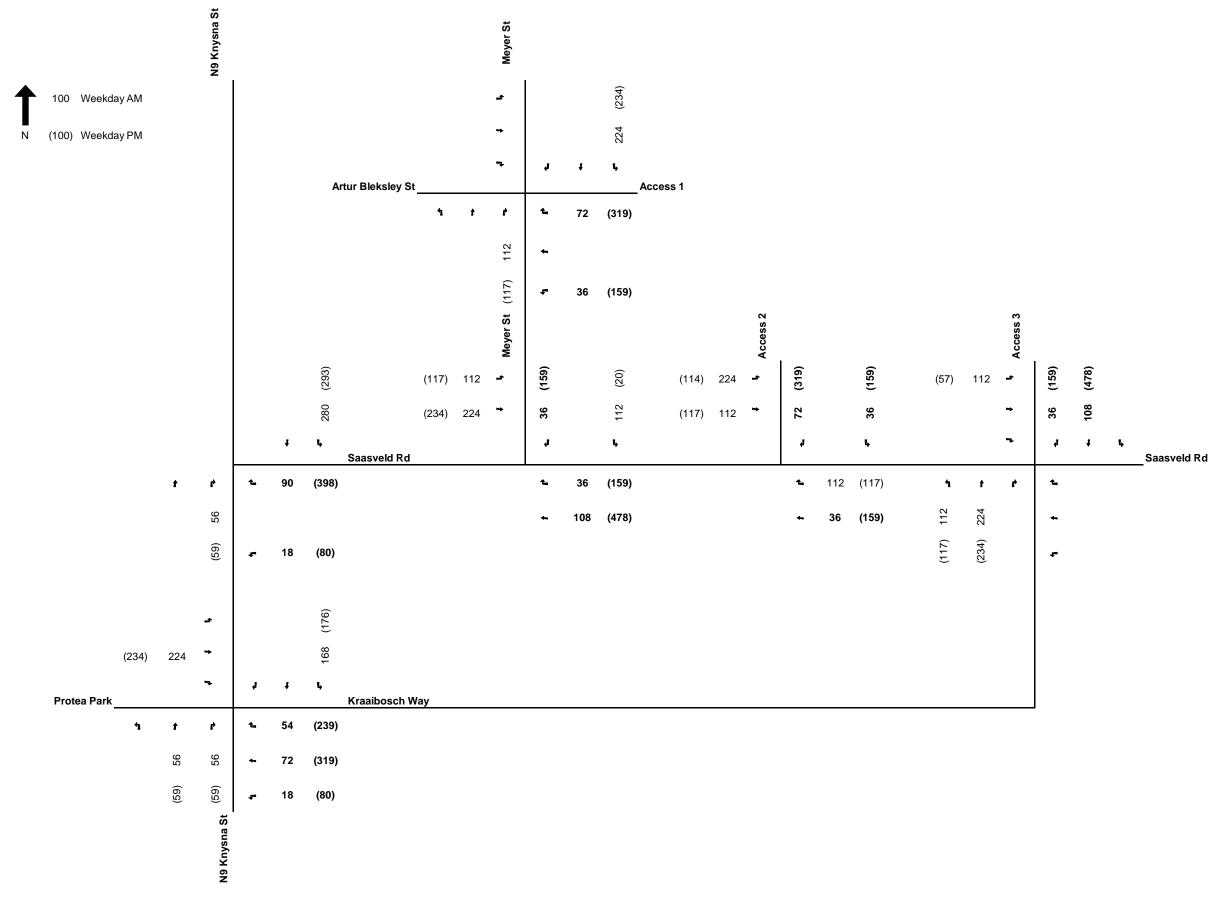


Figure 11 Phase 1+2 Development Trips

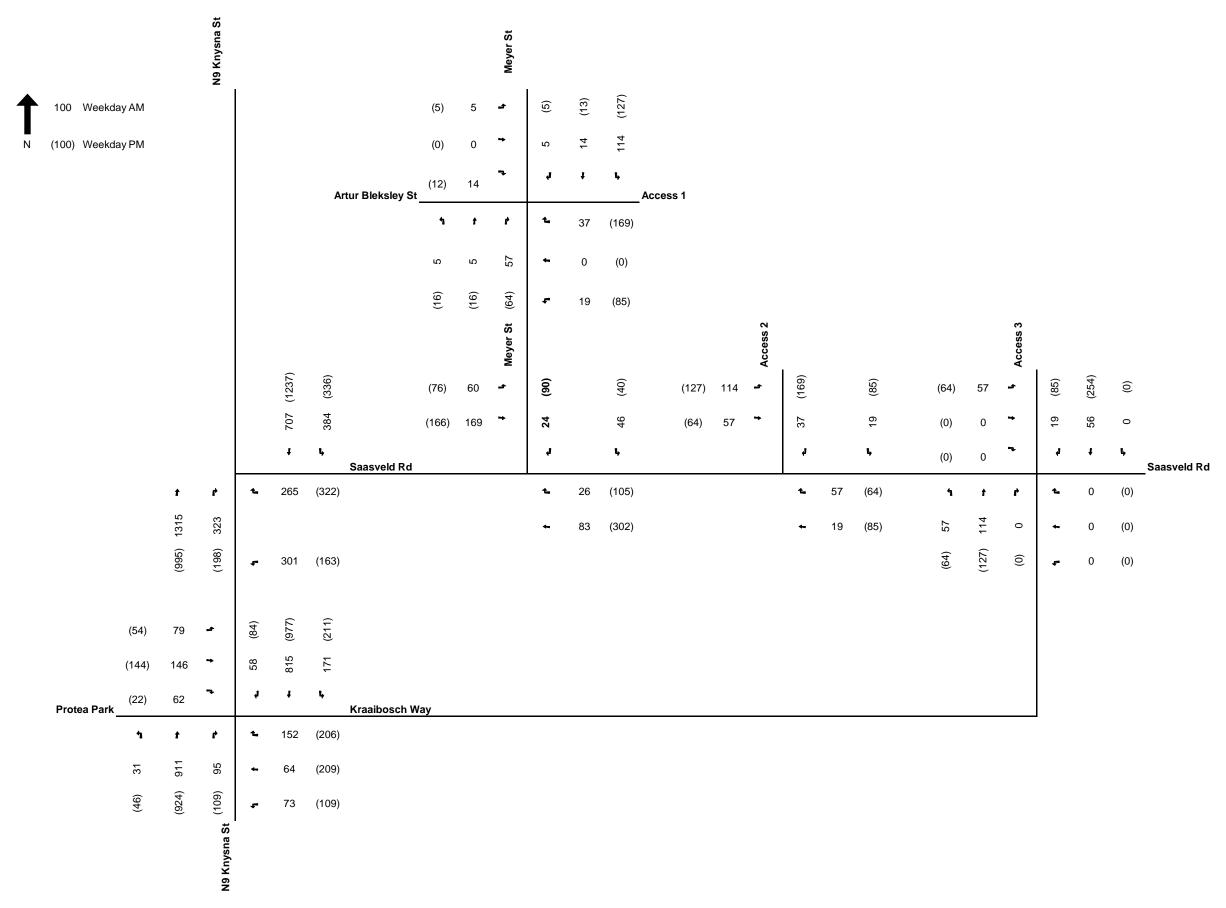


Figure 12 2025 Design Year + Phase 1 Development Trips

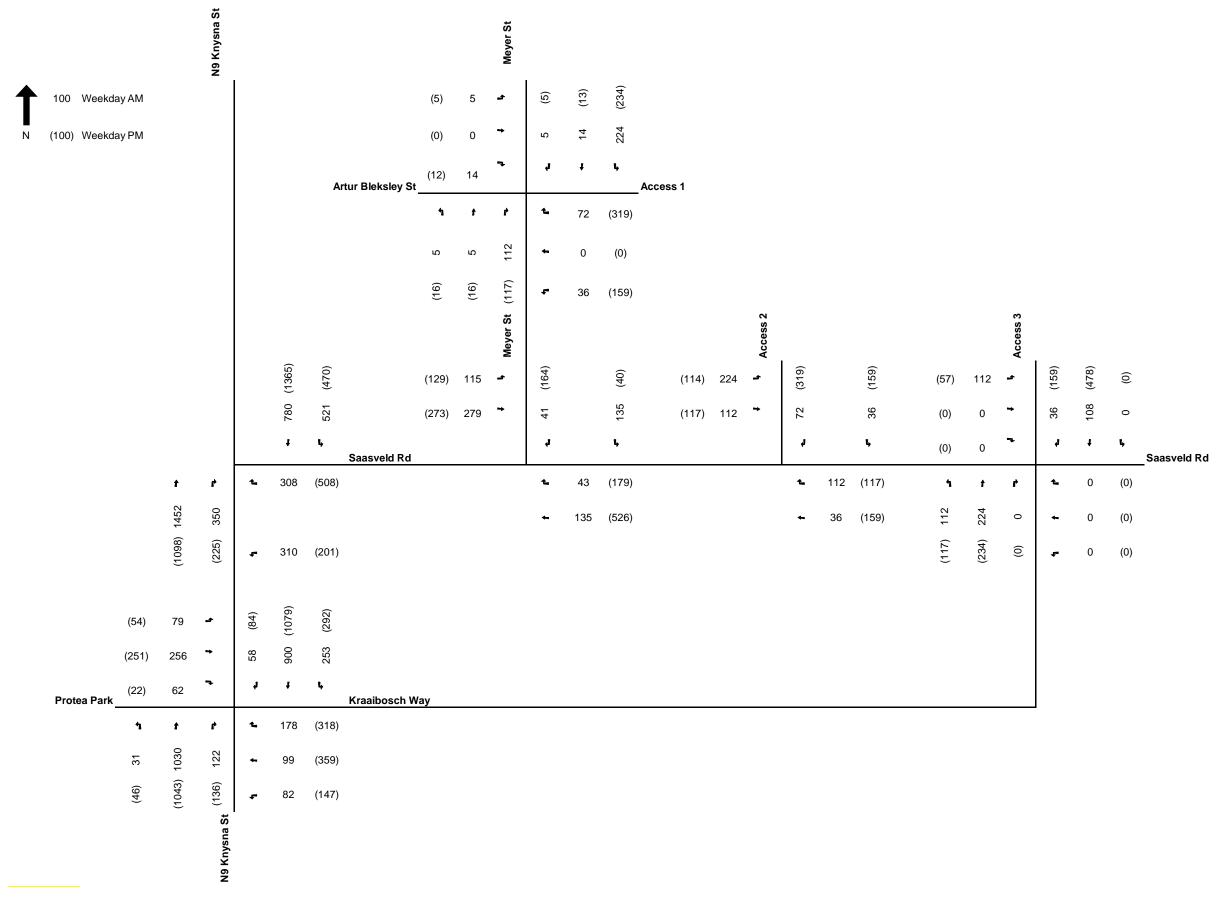


Figure 13 2035 Planning Year + Phase 1+2 Development Trips

8. TRAFFIC IMPACT ASSESSMENT SCENARIOS

The following scenarios were analysed as part of the Traffic Impact Assessment:

- 2019 Base Year Traffic;
- 2024 Design Year + Phase 1 Development Trips;
- 2029 Planning Year + Phase 1+2 Development Trips; and
- 2029 Planning Year + Phase 1+2 Development Trips + Other Development Trips.

The following sub-sections set out the analysis findings.

8.1 Intersection of N9 Knysna Street and Saasveld Road

The intersection of N9 Knysna Street and Saasveld Road is a signalised T-junction. The north approach has a short left-turn slip-lane plus two through lanes, the east approach has a left-turn lane plus a right-turn lane, and the south approach has two through lanes plus a short right-turn lane. Refer to Figure 14.

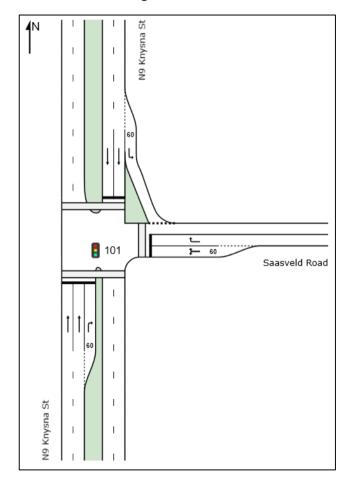


Figure 14 Layout: N9 Knysna Street & Saasveld Road

2019 Base Year Traffic

Taking into consideration the 2019 Base Year traffic flows, the intersection currently operates at Level of Service B during both the Weekday AM and PM Peak Hours, with an average delay of approximately 12 seconds.

<u>2024 Design Year + Phase 1 Development Trips</u>

Taking into consideration the 2024 Design Year plus Phase 1 Development traffic flows, the intersection will continue to operate at Level of Service B during both the Weekday AM and PM Peak Hours, with an average delay of approximately 13 seconds.

2029 Planning Year + Phase 1+2 Development Trips

Taking into consideration the 2029 Design Year plus Phase 1+2 Development traffic flows, the intersection will operate at Level of Service B and C during the Weekday AM and PM Peak Hours, with an average delay of approximately 13 and 22 seconds respectively.

It is concluded that the existing intersection configuration would be suitable to accommodate the anticipated Phase 1+2 Development traffic flows at an acceptable Level of Service by a 2029 Planning Year.

<u>2029 Planning Year + Phase 1+2 Development Trips + Other Development Trips</u>

It is recommended that further intersection analysis be undertaken with consideration of the intersection capacity requirements of the full Kraaibosch Development.

8.2 Intersection of N9 Knysna Street and Kraaibosch Road

The intersection of N9 Knysna Street and Kraaibosch Road is a signalised four-leg intersection. The north approach has a short left-turn slip-lane plus two through lanes plus two short right-turn lanes, the east approach has a short left-turn slip-lane plus two through lanes plus a right-turn lane, the south approach has a short left-turn slip-lane plus two through lanes plus two short right-turn lanes, and the west approach has a short left-turn slip-lane plus two through lanes plus a right-turn lane. Refer to Figure 15.

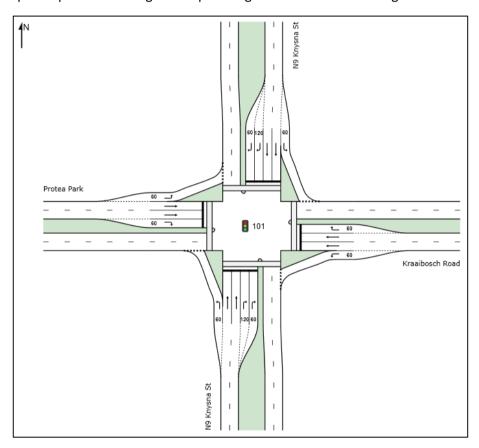


Figure 15 Layout: N9 Knysna Street & Kraaibosch Road

2019 Base Year Traffic

Taking into consideration the 2019 Base Year traffic flows, the intersection currently operates at Level of Service B during both the Weekday AM and PM Peak Hours, with an average delay of approximately 19 seconds.

2024 Design Year + Phase 1 Development Trips

Taking into consideration the 2024 Design Year plus Phase 1 Development traffic flows, the intersection will operate at Level of Service B and C during the Weekday AM and PM Peak Hours, with an average delay of approximately 19 and 23 seconds respectively.

2029 Planning Year + Phase 1+2 Development Trips

Taking into consideration the 2029 Design Year plus Phase 1+2 Development traffic flows, the intersection will operate at Level of Service C during both the Weekday AM and PM Peak Hours, with an average delay of approximately 29 seconds.

It is concluded that the existing intersection configuration would be suitable to accommodate the anticipated Phase 1+2 Development traffic flows at an acceptable Level of Service by a 2029 Planning Year.

<u>2029 Planning Year + Phase 1+2 Development Trips + Other Development Trips</u>

It is recommended that further intersection analysis be undertaken with consideration of the intersection capacity requirements of the full Kraaibosch Development.

8.3 Intersection of Saasveld Road and Meyer Road

The intersection of Saasveld Road and Meyer Road is a priority-controlled T-junction, with Meyer Road being under stop control. The north approach has one lane serving left- and right-turn movements, the east approach has a single lane serving through and right-turn movements, and the west approach has a single lane serving left-turn and through movements. Refer to Figure 16.

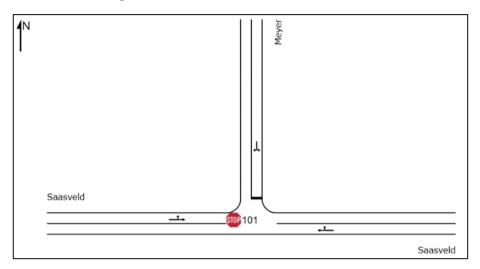


Figure 16 Layout: Saasveld Road & Meyer Road

2019 Base Year Traffic

Taking into consideration the 2019 Base Year traffic flows, the critical movements under stop control currently operate at Level of Service A during both the Weekday AM and PM Peak Hours, with an average delay of approximately 8 seconds.

2024 Design Year + Phase 1 Development Trips

Taking into consideration the 2024 Design Year plus Phase 1 Development traffic flows, the critical movements under stop control will continue to operate at Level of Service A during both the Weekday AM and PM Peak Hours, with an average delay of approximately 9 seconds.

2029 Planning Year + Phase 1+2 Development Trips

Taking into consideration the 2029 Design Year plus Phase 1+2 Development traffic flows, the intersection will operate at Level of Service F during both the Weekday AM and PM Peak Hours, with significant delays being experienced.

It is our submission that intersection upgrades would be required at this point in time, in order to accommodate the anticipated Phase 1+2 Development traffic flows at an acceptable Level of Service. It is proposed to convert the intersection into a roundabout with one circulation lane. Refer to Figure 17.

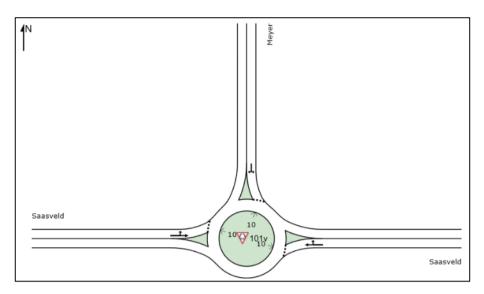


Figure 17 Proposed Layout: Saasveld Road & Meyer Road

Taking into consideration the conversion of the intersection to a roundabout, the critical movements under yield control will operate at Level of Service B during both the Weekday AM and PM Peak Hours, with an average delay of approximately 10 seconds

It is concluded that the proposed intersection configuration would be suitable to accommodate the anticipated Phase 1+2 Development traffic flows at an acceptable Level of Service by a 2029 Planning Year.

8.4 Access 1 and Meyer Road

Access 1 and Meyer Road is planned as a roundabout with one circulating lane. Refer to Figure 18.

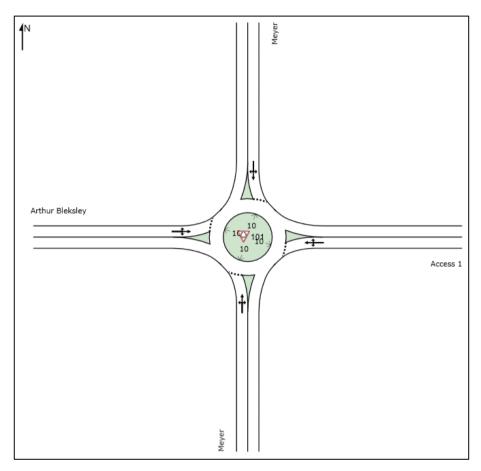


Figure 18 Layout: Access 1 & Meyer Road

2029 Planning Year + Phase 1+2 Development Trips

Taking into consideration the 2029 Design Year plus Phase 1+2 Development traffic flows, the intersection will operate at Level of Service A and B during the Weekday AM and PM Peak Hours, with an average delay of 9 and 10 seconds respectively.

It is concluded that the proposed access configuration would be suitable to accommodate the anticipated Phase 1+2 Development traffic flows at an acceptable Level of Service by a 2029 Planning Year.

8.5 Access 2 and Saasveld Road

Access 2 and Saasveld Road is planned as a roundabout with one circulating lane. Refer to Figure 19.

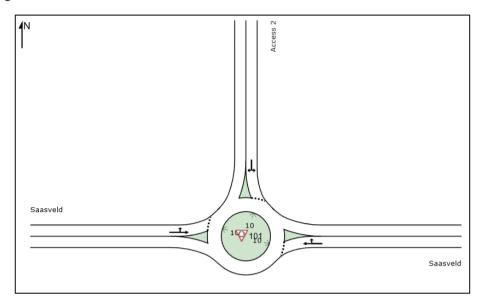


Figure 19 Layout: Access 2 & Saasveld Road

2029 Planning Year + Phase 1+2 Development Trips

Taking into consideration the 2029 Design Year plus Phase 1+2 Development traffic flows, the intersection will operate at Level of Service A and B during the Weekday AM and PM Peak Hours, with an average delay of 9 and 10 seconds respectively.

It is concluded that the proposed access configuration would be suitable to accommodate the anticipated Phase 1+2 Development traffic flows at an acceptable Level of Service by a 2029 Planning Year.

8.6 Access 3 and Saasveld Road / Kraaibosch Road

Access 3 and Saasveld Road / Kraaibosch Road is planned as a roundabout with one circulating lane. Refer to Figure 20.

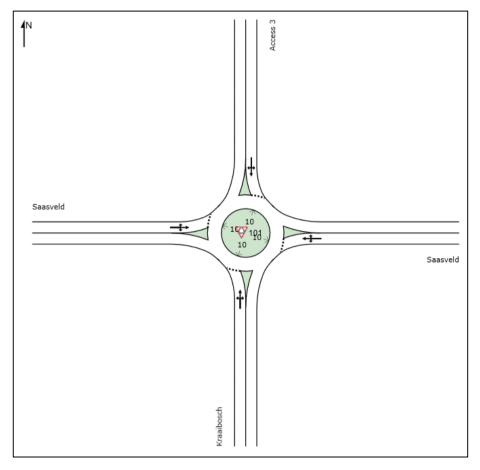


Figure 20 Layout: Access 1 & Meyer Road

2029 Planning Year + Phase 1+2 Development Trips

Taking into consideration the 2029 Design Year plus Phase 1+2 Development traffic flows, the intersection will operate at Level of Service A and B during the Weekday AM and PM Peak Hours, with an average delay of 9 and 12 seconds respectively.

It is concluded that the proposed access configuration would be suitable to accommodate the anticipated Phase 1+2 Development traffic flows at an acceptable Level of Service by a 2029 Planning Year.

8.7 Analysis Summary

A summary of the analysis outputs is provided in Table 12.

Table 12 Analysis Summary (AM / PM)

Scenario	2019 Base Year	2024 Design Year + Phase 1	2029 Design Year + Phase 1+2	2029 Design Year + Phase 1+2 With Upgrades
N9 Knysna Street & Saasveld Road	В / В	В / В	B / C	-
N9 Knysna Street & Kraaibosch Road	В / В	В / С	c / c	-
Saasveld Road & Meyer Road	A / A	A / A	F / F	B / B
Access 1 & Meyer Road	-	-	-	A / B
Access 2 & Saasveld Road	-	-	-	A / B
Access 3 & Saasveld Road / Kraaibosch Road	-	-	-	A / B

9. PROPOSED IMPROVEMENTS

The following transport improvements are proposed as part of the planned development:

2024 Design Year:

N/A.

2029 Planning Year:

 Convert the Saasveld Road & Meyer Road intersection to a roundabout with one circulating lane.

10. SITE TRAFFIC ASSESSMENT

A Site Traffic Assessment did not form part of this project assignment.

11. CONCLUSIONS AND RECOMMENDATIONS

SMEC South Africa (Pty) Ltd was appointed by George Municipality to conduct a Traffic Impact Assessment for the proposed George Campus Development. The site is bound by the Garden Route Dam to the north and Madiba Drive to the south. Refer to Figure 1.

The site measures approximately 118 hectares in extent. The anticipated composition of the development is a Campus catering for 8 000 students, a Waterfront commercial development of 129 300 square metres Gross Lettable Area (GLA), and a Hotel of 34 500 square metres GLA (assumed to be 345 rooms). The Campus component will include residential units for 303 lecturers and 3 009 students.

For the purpose of this TIA it was assumed that the development will be 50% implemented over 5 years by 2024, and 100% within 10 years by 2029It is anticipated that the development will be 100% implemented over 20 years by 2035, with 80% being built out within 10 years by 2025.

George is currently served by three phases of the Geoarge Integrated Public Transport Network (George IPTN). As Kraaibosch and George Campus is rolled out, it is anticipated that these developments will be well served by an extended Phase 1 of the George IPTN. Refer to Figure 4.

The site will be served by three accesses, as follows:

- Access 1 along Stander Street (opposite Arthur Bleksley Street);
- Access 2 along Saasveld Road (300 metres east of Meyer Street); and
- Access 3 along Saasveld Road (600 metres east of Access 2, and opposite Road 1).

The George Campus design focuses on pedestrian accessibility and mobility, providing green corridors linking all components of the development.

It is anticipated that Phase 1 of the planned development would generate 758 and 1 483 new vehicular trips during the Weekday AM and PM Peak Hours respectively, and with Phase 2 it would generate a total of 1 480 and 2 763 new vehicular trips during the Weekday AM and PM Peak Hours respectively.

The following transport improvements are proposed as part of the planned development:

2024 Design Year:

N/A.

2029 Planning Year:

 Convert the Saasveld Road & Meyer Road intersection to a roundabout with one circulating lane.

This Traffic Impact Assessment is supported form a Traffic Engineering point of view, provided that the recommended improvements be implemented in line with appropriate design standards.

ANNEXURE A: TRAFFIC SURVEY DATA

				N9 K	nysna (& Saas	veld								20	19	
							We	ekday	Coun	its							
			1 Peak					Off	Peak						Peak		
		218		292				67		75				110		121	
241	4	+	+	→		156		+	+	→		177	4	+	+	→	
640	→			4	294	1058	→			4	119	1120	→			4	166
	+			+	1191		+			+	1018		+			+	901
	+	4	→	+			+	4	→	+			+	4	→	+	
	Tiı	me			Cauth			Faat	Volun	ne per	Movem	ent		10/224			
Fre	om	7	о .	1	South 2	3	4	East 5	6	7	North 8	9	10	West 11	12	Но	urly
05	:00	05	:15	•			-										
05			:30														
05			:45 :00														
05 06			:15	0	0	0	0	25	2	5	0	5	5	17	0	F.	59
06			:30	0	0	0	0	40	4	6	0	3	0	23	0		35
06			:45	0	0	0	0	63	5	13	0	11	11	57	0		95
06			:00	0	0	0	0	163	63	21	0	44	50	78	0		14
07			:15	0	0	0	0	473 243	158 50	82 81	0	69 20	112 54	121 125	0		670 167
07			:45	0	0	0	0	268	47	73	0	85	39	193	0		712
07	:45	08	:00	0	0	0	0	207	39	56	0	44	36	201	0	2 8	376
08			:15	0	0	0	0	234	37	39	0	36	34	159	0		100
08:			:30 :45	0	0	0	0	155	37 16	26 19	0	18 31	23 33	178	0		264 954
08			:00	0	0	0	0	148 175	16	12	0	21	14	148 160	0		769
09			:15	0	0	0	0	171	22	27	0	11	22	136	0	_	319
09			:30	0	0	0	0	173	14	14	0	13	14	221	0		331
09			:45 :00	0	0	0	0	223	17 20	17 14	0	9 15	26	181	0		709 754
10:			:15	0	0	0	0	175 198	15	20	0	35	24 42	195 263	0		938
10			:30	0	0	0	0	196	17	11	0	17	24	247	0		001
10:			:45	0	0	0	0	235	16	31	0	5	13	189	0)17
10			:00	0	0	0	0	215	22	19	0	16	15	216	0)77 236
11			:15	0	0	0	0	226 252	19 20	26 15	0	18 8	43 8	400 25	0)52
11:			:45	0	0	0	0	241	23	17	0	22	57	366	0		289
11:			:00	0	0	0	0	238	24	12	0	15	22	199	0		296
12			:15	0	0	0	0	265	24	18	0	3	29	222	0		125
12	:15 ·30		:30 :45	0	0	0	0	274 245	48	28 42	0	27 13	48 35	271 194	0		1 93 320
	:45		:00	0	0	0	0	276	42	35	0	20	38	251	0		172
13	:00	13	:15	0	0	0	0	229	35	29	0	23	34	198	0	2 4	159
	:15		:30	0	0	0	0	262	55	6	0	24	50	275	0		135
	:30 :45		:45 :00	0	0	0	0	135 323	27 57	42 18	0	64 6	50 50	229 196	0		129 117
	:00		:15	0	0	0	0	254	41	72	0	63	37	214	0		550
14:	:15	14	:30	0	0	0	0	230	25	39	0	34	42	259	0	2 5	507
	:30		:45	0	0	0	0	220	35	49	0	31	52	275	0		522
	:45 :00		:00 :15	0	0	0	0	250 257	43 28	26 48	0	23 27	50 34	307 200	0		671 584
	:15		:30	0	0	0	0	177	22	16	0	5	29	262	0		166
15	:30		:45	0	0	0	0	188	30	33	0	47	39	185	0		326
	:45		:00	0	0	0	0	252	43	45	0	44	37	243	0		291
	:00 :15		:15	0	0	0	0	291	27	47	0	35	24	156	0		277 139
	:15		:45	0	0	0	0	196 221	27 44	21 37	0	39 23	62 39	328 240	0		521
	:45		:00	0	0	0	0	248	45	40	0	27	43	283	0		543
	:00		:15	0	0	0	0	236	50	23	0	21	33	269	0		595
	:15		:30	0	0	0	0	242	41	31	0	40	49	269	0		594
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3		+	+	→		5	1	+	+	→		12		+	*	→	
55	→			1	7	29	→				5	39	→			1	20
	+			+	27		+			+	18		+			+	48
	+	1	→	+			+	1	→	-			+	1	→	+	
	Tiı	me				<u> </u>			Volu	ne per	Movem	nent					
Fre	om	١ ١	Го		South			East			North			West		Но	urly
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	:30		:45	0	0	0	0	3	1	1	0	1	0	2	0		4
06	:45	07	:00	0	0	0	0	5	2	3	0	3	0	7	0		34
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	:45		:00	0	0	0	0	5	1	7	0	1	1	16	0		18
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	:15		:30	0	0	0	0	8	2	1	0	0	1	1	0		66
	:30 :45		:45	0	0	0	0	5	1	2	0	0	1	2 5	0		54 51
	:00		:15	0	0	0	0	6 2	2	7	0	0	1	າ 11	0	_	62
	:15		:30	0	0	0	0	5	4	1	0	0	0	3	0		52
	:30		:45	0	0	0	0	2	0	4	0	0	0	6	0		33
	:45 :00		:00	0	0	0	0	3	2	3	0	1	0	5 3	0		51 52
	:15		:30	0	0	0	0	6	2	3	0	2	0	7	0		59
	:30		:45	0	0	0	0	6	2	5	0	0	1	4	0		35
	:45		:00	0	0	0	0	0	3	1	0	0	0	2	0		8
	:00		::15	0	0	0	0	3	0	7	0	1	2	2 4	0		57 18
	:30		:45	0	0	0	0	1	3	2	0	2	1	3	0		12
12	:45	13	:00	0	0	0	0	5	4	4	0	0	1	3	0	5	53
	:00		:15	0	0	0	0	3	2	2	0	0	1	4	0		52
	:15		:30	0	0	0	0	4	1	2	0	0	3	7 6	0		58 53
	:45		:00	0	0	0	0	7	1	4	0	1	0	12	0		7 1
	:00		:15	0	0	0	0	7	6	4	0	2	3	6	0		37
	:15		:30 :45	0	0	0	0	4	3	3	0	0	1	11	0		92
	:30 :45		:45	0	0	0	0	2	2	4	0	1	1	4 5	0		37 77
	:00	_	:15	0	0	0	0	7	2	4	0	3	3	4	0		72
	:15		:30	0	0	0	0	4	3	2	0	2	2	4	0		37
	:30 :45		:45	0	0	0	0	7	3	5 5	0	0	1	6 11	0		76 37
	:00		:15	0	0	0	0	8	2	3	0	0	5	6	0		38
16	:15	16	:30	0	0	0	0	19	7	6	0	1	2	9	0	1	15
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	:30		:45	0	0	0	0	3	1	8	0	0	0	13	0		12
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				N9 Kn	ysna &	Kraai	bosch	· · · · ·							20	19	
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		ΑN	/I Peak						Peak					PM I	Peak		
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85		+	•	→		132	•	+	•	→		116		+	•	→	
738		_			66	944	<u> </u>		_	-	87	885		_			77
58	→			4	799	100	7				914	84	→			4	808
56	4	_		+	31	100		4	→	+	41	04	+			+	46
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06	:15	06	:30	1	2	14	2	45	5	6	5	5	2	28	3	1	69
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	:00		:30	22	9	15 21	6 10	238 226	13 9	16 16	10 5	37 46	7 23	131 191	10 12		137 504
	:30		:45	18	8	17	6	187	19	14	7	21	38	237	20		026
	:45		:00	19	9	9	9	148	25	18	5	20	17	179	16		165
	:00		:15	11	6	6	5	155	14	11	5	23	17	166	17		087
	:15		:30	17	6	11	6	170	14	11	2	29	15	170	12		965 307
	:45		:00	12 5	3 5	12 4	6 5	132 100	11 5	18 6	8 7	30 18	29 22	162 145	11 19		674
	:00	_	:15	13	3	14	6	136	13	11	4	14	14	157	20	_	643
09	:15	09	:30	19	6	6	4	128	13	10	9	20	14	154	20		583
	:30		:45	13	6	7	6	152	16	11	5	26	19	185	19		314
	:45 :00	_	:00	20 9	5 1	5 11	3	171 144	25	22	5 1	23 14	38 19	229 175	34		353 378
	:15		:30	8	5	11	9	203	16 13	19	8	28	24	207	15 34)44
	:30		:45	14	8	10	13	192	13	15	9	28	19	217	23	2 1	140
	:45		:00	31	6	8	12	263	25	31	11	42	43	229	31		292
	:00		:15	8	2	1	3	132	13	11	1	6	15	109	10		173
	:15		:30 :45	8 16	7	7	9	119 145	8 16	15 9	6	9 11	22 13	115 125	16 6		937 735
	:45		:00	16	18	11	11	276	21	16	4	21	49	395	49		390
	:00	12	::15	5	6	6	7	173	18	15	5	11	33	147	11		016
	:15		::30	29	6	5	12	309	30	15	7	25	22	215	24		382
	:30		:45	14	5	8	11	156	18	16	4	8	28	187	16		194
	:45 :00		:00	9 17	7	7	4 9	254 199	30 12	19 15	9 10	18 14	27 30	209 172	25 28		217 300
	:15		:30	28	7	7	8	228	19	20	6	12	29	243	25		233
	:30		:45	16	3	7	8	220	20	14	4	13	28	146	11		252
	:45	_	:00	16	8	6	4	225	22	8	3	19	18	183	17		171
	:00 :15		:15 :30	24 19	8 2	7	12 10	204	16	7 10	7	29 16	20	208	25		218 143
	:15		:45	30	2	7	11	192 221	13 32	10	8 5	35	25 43	228 289	27 41		381
	:45		:00	15	5	7	6	192	19	6	4	17	18	143	13		297
	:00		:15	14	9	12	2	188	19	11	5	25	23	224	41		303
	:15		:30	6	4	7	11	194	23	13	5	22	22	175	21		249
	:30 :45		:45	16 11	10	5 7	5 11	173 201	23 18	9	1 4	16	13 23	176 188	18 17		972 951
	:45	_	6:15	17	4	3	15	247	23	16	12	25 22	41	254	22		154
	:15		:30	12	5	6	8	138	11	15	5	13	19	169	8		060
	:30		:45	15	6	6	14	211	20	22	12	21	26	215	23		200
	:45		:00	10	2	7	9	212	23	14	11	23	30	247	31		295
	:00 :15		':15 ':30	8 5	9	17 10	19 6	181 97	16 10	13 6	5 4	13 5	31 6	209	17 11		157 998
	:30		:45	32	3 5	27	29	296	32	26	1	32	32	87 312	30		261
	:45		:00	14	1	5	5	137	15	4	0	11	22	131	8		995
	:00		:15														157
18	:15	18	:30													1 2	207

ANNEXURE B: DETAILED SIDRA OUTPUTS

Intersection of N9 Knysna Street & Saasveld Road

2019 Base Year

AM Peak Hour

Move	ment	Performa	ance -	Vehic	les							
Mov ID	Turn	Demand Total	Flows HV	Deg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed
		veh/h	%	v/c	sec		veh	m				km/h
South:	: N9 Kı	nysna St										
2	T1	1254	3.0	0.566	7.1	LOS A	8.5	60.7	0.72	0.64	0.72	53.7
3	R2	309	3.0	0.594	13.9	LOS B	3.8	27.1	0.91	0.81	0.93	47.7
Appro	ach	1563	3.0	0.594	8.4	LOS A	8.5	60.7	0.76	0.67	0.77	52.4
East: \$	Saasv	eld Road										
4	L2	307	3.0	0.681	22.8	LOS C	6.1	43.8	0.97	0.87	1.09	42.7
6	R2	229	3.0	0.509	21.1	LOS C	4.2	29.9	0.92	0.80	0.92	43.6
Appro	ach	537	3.0	0.681	22.1	LOS C	6.1	43.8	0.95	0.84	1.02	43.1
North:	N9 Kr	nysna St										
7	L2	254	3.0	0.210	7.5	LOS A	1.2	8.8	0.45	0.67	0.45	52.6
8	T1	674	3.0	0.639	15.7	LOS B	6.3	45.5	0.94	0.82	1.01	47.7
Appro	ach	927	3.0	0.639	13.4	LOS B	6.3	45.5	0.81	0.78	0.85	49.0
All Vel	nicles	3027	3.0	0.681	12.4	LOS B	8.5	60.7	0.81	0.73	0.84	49.5

Move	ment	Performa	ince -	Vehic	les							
Mov	Turn	Demand	Flows	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective	Aver. No.	Average
ID	Tulli	Total	HV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Cycles	Speed
		veh/h	%	v/c	sec		veh	m				km/h
South	: N9 Kı	nysna St										
2	T1	948	3.0	0.351	4.5	LOS A	5.4	38.7	0.50	0.43	0.50	55.9
3	R2	175	3.0	0.394	13.4	LOS B	1.8	13.2	0.82	0.77	0.82	48.0
Appro	ach	1123	3.0	0.394	5.9	LOS A	5.4	38.7	0.55	0.49	0.55	54.5
East: \$	Saasve	eld Road										
4	L2	127	3.0	0.397	27.1	LOS C	3.0	21.3	0.94	0.77	0.94	40.7
6	R2	116	3.0	0.361	27.0	LOS C	2.7	19.2	0.93	0.77	0.93	40.8
Appro	ach	243	3.0	0.397	27.1	LOS C	3.0	21.3	0.94	0.77	0.94	40.7
North:	N9 Kr	nysna St										
7	L2	186	3.0	0.139	6.9	LOS A	0.8	5.8	0.33	0.64	0.33	53.0
8	T1	1179	3.0	0.666	12.9	LOS B	11.9	85.2	0.86	0.76	0.86	49.5
Appro	ach	1365	3.0	0.666	12.1	LOS B	11.9	85.2	0.79	0.74	0.79	50.0
All Vel	hicles	2732	3.0	0.666	10.9	LOS B	11.9	85.2	0.70	0.64	0.70	50.7

2024 Design Year + Phase 1 Development

AM Peak Hour

Move	ment	Performa	nce -	Vehic	les							
Mov ID	Turn	Demand Total	Flows HV	Deg. Satn	Average	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Aver. No. Cvcles	Average Speed
		veh/h	%	v/c	sec	OCIVICC	vernicies	Distance	Queucu	Otop Itale	Oyolos	km/h
South	: N9 Kı	nysna St	/0	V/C	360		Ven					KIII/II
2	T1	1384	3.0	0.625	7.4	LOS A	9.8	70.2	0.76	0.67	0.76	53.5
3	R2	340	3.0	0.672	15.0	LOS B	4.4	31.6	0.95	0.85	1.05	47.0
Appro	ach	1724	3.0	0.672	8.9	LOS A	9.8	70.2	0.80	0.71	0.82	52.1
East:	Saasv	eld Road										
4	L2	317	3.0	0.702	23.1	LOS C	6.4	45.8	0.97	0.89	1.13	42.6
6	R2	279	3.0	0.618	21.9	LOS C	5.3	38.2	0.95	0.84	1.01	43.2
Appro	ach	596	3.0	0.702	22.6	LOS C	6.4	45.8	0.96	0.86	1.07	42.9
North:	N9 Kr	nysna St										
7	L2	404	3.0	0.333	7.7	LOS A	2.1	15.2	0.49	0.69	0.49	52.4
8	T1	744	3.0	0.706	16.7	LOS B	7.4	52.8	0.96	0.88	1.10	47.1
Appro	ach	1148	3.0	0.706	13.5	LOS B	7.4	52.8	0.80	0.81	0.88	48.8
All Vel	hicles	3468	3.0	0.706	12.8	LOS B	9.8	70.2	0.82	0.77	0.88	49.2

Move	ment	Performa	nce -	Vehic	les							
Mov	Turn	Demand	Flows	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective	Aver. No.	Average
ID	Tuiti	Total	HV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Cycles	Speed
		veh/h	%	v/c	sec		veh	m				km/h
South	: N9 K	nysna St										
2	T1	1047	3.0	0.401	5.2	LOS A	6.5	46.6	0.54	0.47	0.54	55.3
3	R2	208	3.0	0.500	15.0	LOS B	2.5	17.8	0.90	0.79	0.90	47.0
Appro	ach	1256	3.0	0.500	6.8	LOS A	6.5	46.6	0.60	0.53	0.60	53.7
East:	Saasv	eld Road										
4	L2	172	3.0	0.707	28.9	LOS C	6.5	46.6	0.99	0.88	1.14	39.9
6	R2	339	3.0	0.707	28.9	LOS C	6.5	46.6	0.99	0.88	1.14	39.9
Appro	ach	511	3.0	0.707	28.9	LOS C	6.5	46.6	0.99	0.88	1.14	39.9
North:	N9 Kr	nysna St										
7	L2	354	3.0	0.264	7.2	LOS A	1.7	12.4	0.38	0.66	0.38	52.8
8	T1	1302	3.0	0.772	16.7	LOS B	15.4	110.7	0.93	0.89	1.05	47.1
Appro	ach	1656	3.0	0.772	14.7	LOS B	15.4	110.7	0.81	0.84	0.90	48.2
All Ve	hicles	3422	3.0	0.772	13.9	LOS B	15.4	110.7	0.76	0.73	0.83	48.5

2029 Planning Year + Phase 1+2 Development

AM Peak Hour

Move	ement	Performa	nce -	Vehic	les							
Mov	Turn	Demand I	Flows	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective	Aver. No.	Average
ID	Tulli	Total	HV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Cycles	Speed
		veh/h	%	v/c	sec		veh	m				km/h
South	: N9 K	nysna St										
2	T1	1528	3.0	0.690	8.2	LOS A	11.7	83.9	0.80	0.73	0.82	52.9
3	R2	368	3.0	0.748	16.5	LOS B	5.1	36.7	0.98	0.90	1.19	46.1
Appro	ach	1897	3.0	0.748	9.8	LOS A	11.7	83.9	0.84	0.76	0.89	51.4
East:	Saasv	eld Road										
4	L2	326	3.0	0.723	23.5	LOS C	6.7	47.9	0.98	0.90	1.16	42.4
6	R2	324	3.0	0.719	23.5	LOS C	6.6	47.5	0.98	0.90	1.15	42.5
Appro	ach	651	3.0	0.723	23.5	LOS C	6.7	47.9	0.98	0.90	1.16	42.4
North	: N9 Kr	nysna St										
7	L2	548	3.0	0.448	8.1	LOS A	3.2	23.2	0.54	0.71	0.54	52.1
8	T1	821	3.0	0.779	18.5	LOS B	8.7	62.5	0.99	0.96	1.24	46.0
Appro	ach	1369	3.0	0.779	14.3	LOS B	8.7	62.5	0.81	0.86	0.96	48.3
All Ve	hicles	3917	3.0	0.779	13.7	LOS B	11.7	83.9	0.85	0.82	0.96	48.6

Move	ment	Performa	nce -	Vehic	les							
Mov	Turn	Demand Total	Flows HV	Deg. Satn		Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed
		veh/h	%	v/c	sec	0011100	vernicles	m	Queucu	Ctop reace	O y o loo	km/h
South	: N9 K	nysna St										
2	T1	1156	3.0	0.414	6.7	LOS A	10.4	74.4	0.50	0.45	0.50	54.1
3	R2	237	3.0	0.613	24.3	LOS C	5.5	39.5	0.97	0.84	1.02	42.0
Appro	ach	1393	3.0	0.613	9.7	LOS A	10.4	74.4	0.58	0.51	0.59	51.5
East:	Saasv	eld Road										
4	L2	212	3.0	0.876	48.1	LOS D	16.8	120.8	1.00	0.99	1.33	33.0
6	R2	535	3.0	0.876	48.1	LOS D	16.8	120.8	1.00	0.99	1.33	33.0
Appro	ach	746	3.0	0.876	48.1	LOS D	16.8	120.8	1.00	0.99	1.33	33.0
North:	N9 Kr	nysna St										
7	L2	495	3.0	0.363	8.2	LOS A	4.5	32.5	0.41	0.68	0.41	52.2
8	T1	1437	3.0	0.861	27.4	LOS C	33.9	243.3	0.90	0.93	1.07	41.4
Appro	ach	1932	3.0	0.861	22.5	LOS C	33.9	243.3	0.77	0.87	0.90	43.7
All Ve	hicles	4071	3.0	0.876	22.8	LOS C	33.9	243.3	0.75	0.77	0.87	43.4

Intersection of N9 Knysna Street & Kraaibosch Road

2019 Base Year AM Peak Hour

Move	ement F	Performanc	e - Ve	hicles								
Mov	Turn	Demand F	lows	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective	Aver. No.	Average
ID	Tulli	Total	HV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Cycles	Speed
		veh/h	%	v/c	sec		veh	m				km/h
South	: N9 Kn	ysna St										
1	L2	33	3.0	0.023	6.1	LOS A	0.1	0.7	0.17	0.58	0.17	53.6
2	T1	841	3.0	0.558	19.7	LOS B	11.7	83.8	0.86	0.74	0.86	45.4
3	R2	69	3.0	0.088	17.1	LOS B	0.6	4.5	0.74	0.69	0.74	46.2
Appro	ach	943	3.0	0.558	19.0	LOS B	11.7	83.8	0.82	0.73	0.82	45.7
East:	Kraaibo	sch Road										
4	L2	67	3.0	0.058	8.3	LOS A	0.6	4.3	0.36	0.63	0.36	52.0
5	T1	28	3.0	0.039	26.4	LOS C	0.4	2.9	0.86	0.60	0.86	42.0
6	R2	131	3.0	0.294	24.8	LOS C	3.4	24.4	0.83	0.74	0.83	42.3
Appro	ach	226	3.0	0.294	20.1	LOS C	3.4	24.4	0.70	0.69	0.70	44.7
North:	N9 Kn	ysna St										
7	L2	89	3.0	0.060	6.1	LOS A	0.3	2.0	0.17	0.59	0.17	53.6
8	T1	777	3.0	0.516	19.3	LOS B	10.6	75.9	0.84	0.72	0.84	45.7
9	R2	61	3.0	0.084	17.4	LOS B	0.5	3.9	0.76	0.69	0.76	46.0
Appro	ach	927	3.0	0.516	17.9	LOS B	10.6	75.9	0.77	0.71	0.77	46.3
West:	Protea	Park										
10	L2	83	3.0	0.085	8.9	LOS A	0.8	6.0	0.40	0.64	0.40	51.6
11	T1	34	3.0	0.051	26.6	LOS C	0.5	3.5	0.86	0.61	0.86	41.9
12	R2	65	3.0	0.419	41.0	LOS D	2.3	16.4	0.99	0.75	0.99	35.7
Appro	ach	182	3.0	0.419	23.7	LOS C	2.3	16.4	0.69	0.68	0.69	42.9
All Ve	hicles	2279	3.0	0.558	19.0	LOS B	11.7	83.8	0.78	0.71	0.78	45.6

Move	ment F	Performanc	e - Ve	hicles								
Mov	Turn	Demand F	lows	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective	Aver. No.	Average
ID	Tuiti	Total	HV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Cycles	Speed
		veh/h	%	v/c	sec		veh	m				km/h
South	: N9 Kn	ysna St										
1	L2	48	3.0	0.035	6.1	LOS A	0.2	1.2	0.13	0.58	0.13	53.7
2	T1	851	3.0	0.370	15.1	LOS B	12.8	91.7	0.61	0.54	0.61	48.2
3	R2	81	3.0	0.107	15.7	LOS B	0.8	6.0	0.57	0.67	0.57	47.1
Appro	ach	980	3.0	0.370	14.7	LOS B	12.8	91.7	0.58	0.55	0.58	48.3
East: I	Kraaibo	sch Road										
4	L2	71	3.0	0.064	7.9	LOS A	0.7	5.2	0.26	0.61	0.26	52.4
5	T1	42	3.0	0.065	42.9	LOS D	1.0	7.0	0.88	0.63	0.88	35.4
6	R2	83	3.0	0.240	41.9	LOS D	3.7	26.5	0.86	0.73	0.86	35.4
Appro	ach	196	3.0	0.240	29.9	LOS C	3.7	26.5	0.65	0.67	0.65	40.0
North:	N9 Kn	ysna St										
7	L2	122	3.0	0.082	6.0	LOS A	0.4	2.8	0.12	0.58	0.12	53.7
8	T1	932	3.0	0.405	15.4	LOS B	14.3	102.9	0.63	0.55	0.63	47.9
9	R2	88	3.0	0.114	15.3	LOS B	0.9	6.6	0.55	0.66	0.55	47.3
Appro	ach	1142	3.0	0.405	14.4	LOS B	14.3	102.9	0.57	0.56	0.57	48.5
West:	Protea	Park										
10	L2	57	3.0	0.057	8.1	LOS A	0.6	4.4	0.27	0.61	0.27	52.2
11	T1	18	3.0	0.030	42.6	LOS D	0.4	2.9	0.87	0.60	0.87	35.5
12	R2	23	3.0	0.233	62.8	LOS E	1.3	9.1	0.99	0.71	0.99	29.5
Appro	ach	98	3.0	0.233	27.3	LOS C	1.3	9.1	0.55	0.63	0.55	41.2
All Vel	hicles	2416	3.0	0.405	16.3	LOS B	14.3	102.9	0.58	0.57	0.58	47.3

2024 Design Year + Phase 1 Development AM Peak Hour

Move	ment F	Performance	e - Ve	hicles								
Mov	Turn	Demand F	lows	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective	Aver. No.	Average
ID	Turn	Total	HV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Cycles	Speed
		veh/h	%	v/c	sec		veh	m				km/h
South:	: N9 Kn	ysna St										
1	L2	33	3.0	0.023	6.2	LOS A	0.1	0.8	0.18	0.58	0.18	53.5
2	T1	959	3.0	0.637	20.4	LOS C	13.8	99.1	0.89	0.77	0.89	45.0
3	R2	100	3.0	0.135	17.6	LOS B	0.9	6.5	0.77	0.70	0.77	45.9
Appro	ach	1092	3.0	0.637	19.7	LOS B	13.8	99.1	0.86	0.76	0.86	45.3
East: I	Kraaibo	sch Road										
4	L2	77	3.0	0.068	8.6	LOS A	0.7	5.2	0.38	0.64	0.38	51.8
5	T1	67	3.0	0.093	26.9	LOS C	1.0	7.1	0.87	0.64	0.87	41.8
6	R2	160	3.0	0.406	25.4	LOS C	4.2	30.5	0.88	0.77	0.88	42.0
Appro	ach	304	3.0	0.406	21.5	LOS C	4.2	30.5	0.75	0.70	0.75	44.1
North:	N9 Kn	ysna St										
7	L2	180	3.0	0.129	6.6	LOS A	0.9	6.5	0.24	0.61	0.24	53.3
8	T1	858	3.0	0.570	19.8	LOS B	12.0	85.9	0.86	0.74	0.86	45.4
9	R2	61	3.0	0.090	18.0	LOS B	0.5	3.9	0.79	0.69	0.79	45.7
Appro	ach	1099	3.0	0.570	17.5	LOS B	12.0	85.9	0.75	0.72	0.75	46.5
West:	Protea	Park										
10	L2	83	3.0	0.091	10.0	LOS A	1.0	7.0	0.44	0.66	0.44	50.8
11	T1	154	3.0	0.234	27.9	LOS C	2.3	16.8	0.90	0.69	0.90	41.3
12	R2	65	3.0	0.419	41.0	LOS D	2.3	16.4	0.99	0.75	0.99	35.7
Appro	ach	302	3.0	0.419	25.8	LOS C	2.3	16.8	0.79	0.70	0.79	42.0
All Vel	nicles	2797	3.0	0.637	19.7	LOS B	13.8	99.1	0.80	0.73	0.80	45.3

			1.6									
	ment F	Performanc					050/ D					
Mov	Turn	Demand F		Deg.	Average		95% Back		Prop.		Aver. No.	
ID		Total	HV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Cycles	Speed
		veh/h	%	v/c	sec		veh	m				km/h
South		ysna St										
1	L2	48		0.035	6.6	LOS A	0.2	1.8	0.23	0.60	0.23	53.3
2	T1	973	3.0	0.734	24.6	LOS C	15.6	112.0	0.95	0.86	1.01	42.8
3	R2	115	3.0	0.186	20.8	LOS C	1.1	8.2	0.88	0.73	0.88	44.2
Appro	ach	1136	3.0	0.734	23.5	LOS C	15.6	112.0	0.91	0.84	0.96	43.3
East: I	Kraaibo	sch Road										
4	L2	115	3.0	0.102	9.3	LOS A	1.2	8.9	0.42	0.66	0.42	51.3
5	T1	220	3.0	0.304	28.2	LOS C	3.4	24.4	0.92	0.72	0.92	41.2
6	R2	217	3.0	0.451	23.2	LOS C	5.4	38.9	0.87	0.78	0.87	43.1
Appro	ach	552	3.0	0.451	22.3	LOS C	5.4	38.9	0.79	0.73	0.79	43.7
North:	N9 Kny	ysna St										
7	L2	222	3.0	0.157	6.6	LOS A	1.2	8.3	0.24	0.61	0.24	53.3
8	T1	1028	3.0	0.792	26.9	LOS C	18.0	129.1	0.97	0.93	1.09	41.7
9	R2	88	3.0	0.142	20.3	LOS C	0.9	6.3	0.85	0.71	0.85	44.4
Appro	ach	1339	3.0	0.792	23.1	LOS C	18.0	129.1	0.84	0.86	0.94	43.4
West:	Protea	Park										
10	L2	57	3.0	0.065	10.8	LOS B	0.7	5.1	0.48	0.65	0.48	50.3
11	T1	152	3.0	0.231	27.9	LOS C	2.3	16.6	0.90	0.69	0.90	41.3
12	R2	23	3.0	0.099	35.8	LOS D	0.7	5.2	0.91	0.70	0.91	37.6
Appro	ach	232	3.0	0.231	24.5	LOS C	2.3	16.6	0.80	0.68	0.80	42.8
All Vel	nicles	3258	3.0	0.792	23.2	LOS C	18.0	129.1	0.85	0.82	0.91	43.4

2029 Planning Year + Phase 1+2 Development AM Peak Hour

Move	ment F	Performanc	e - Ve	hicles								
Mov		Demand F		Deg.	Average	I evel of	95% Back	of Queue	Prop.	Effective	Aver. No.	Average
ID	Turn	Total	HV	Satn		Service	Vehicles	Distance	Queued	Stop Rate	Cycles	Speed
		veh/h	%	v/c	sec		veh	m				km/h
South:	: N9 Kn	ysna St	-									
1	L2	33	3.0	0.023	6.2	LOS A	0.1	0.8	0.18	0.58	0.18	53.5
2	T1	1084	3.0	0.722	21.9	LOS C	16.7	119.6	0.93	0.83	0.96	44.2
3	R2	128	3.0	0.185	18.4	LOS B	1.2	8.5	0.81	0.72	0.81	45.5
Appro	ach	1245	3.0	0.722	21.2	LOS C	16.7	119.6	0.89	0.81	0.92	44.5
East: I	Kraaibo	sch Road										
4	L2	86	3.0	0.078	9.3	LOS A	0.9	6.6	0.41	0.65	0.41	51.3
5	T1	104	3.0	0.144	27.2	LOS C	1.6	11.1	0.88	0.66	0.88	41.6
6	R2	187	3.0	0.531	26.1	LOS C	5.1	36.3	0.93	0.78	0.93	41.7
Appro	ach	378	3.0	0.531	22.6	LOS C	5.1	36.3	0.80	0.72	0.80	43.5
North:	N9 Kn	ysna St										
7	L2	266	3.0	0.199	7.1	LOS A	1.8	12.6	0.30	0.64	0.30	52.9
8	T1	947	3.0	0.629	20.3	LOS C	13.6	97.5	0.89	0.77	0.89	45.1
9	R2	61	3.0	0.096	18.7	LOS B	0.5	3.9	0.82	0.70	0.82	45.3
Appro	ach	1275	3.0	0.629	17.5	LOS B	13.6	97.5	0.76	0.74	0.76	46.5
West:	Protea	Park										
10	L2	83	3.0	0.096	11.3	LOS B	1.1	8.0	0.51	0.67	0.51	49.9
11	T1	269	3.0	0.411	29.0	LOS C	4.3	30.7	0.94	0.75	0.94	40.8
12	R2	65	3.0	0.419	41.0	LOS D	2.3	16.4	0.99	0.75	0.99	35.7
Appro	ach	418	3.0	0.419	27.3	LOS C	4.3	30.7	0.86	0.73	0.86	41.4
All Vel	hicles	3316	3.0	0.722	20.7	LOS C	16.7	119.6	0.83	0.76	0.84	44.7

urn	e <mark>rformanc</mark> Demand F		nicles								
urn	Demand F										_
			Deg.	Average		95% Back		Prop.		Aver. No.	
	Total	HV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Cycles	Speed
	veh/h	%	v/c	sec		veh	m				km/h
9 Kny	sna St										
L2	48	3.0	0.036	7.1	LOS A	0.4	2.7	0.23	0.60	0.23	53.0
T1	1098	3.0	0.717	28.8	LOS C	23.4	168.0	0.91	0.81	0.91	40.8
R2	143	3.0	0.322	28.4	LOS C	2.0	14.5	0.92	0.75	0.92	40.5
า	1289	3.0	0.717	28.0	LOS C	23.4	168.0	0.89	0.79	0.89	41.1
.,											
											40.0
			-			_	_				49.2
						_					35.4
R2											40.4
า	867	3.0	0.608	32.0	LOS C	11.6	83.5	0.85	0.78	0.85	39.2
a Kny	ena St										
•		3.0	0.210	73	108 4	27	10.2	0.27	0.63	0.27	52.8
			-	_				•			38.8
							_			-	41.5
											41.2
	1332	3.0	0.010	21.0	LO3 C	30.0	213.1	0.00	0.03	0.00	41.2
otea F	Park										
L2	57	3.0	0.076	14.8	LOS B	1.1	8.2	0.52	0.67	0.52	47.8
T1	264	3.0	0.432	41.9	LOS D	6.0	42.9	0.95	0.76	0.95	35.7
R2	23	3.0	0.064	40.7	LOS D	0.9	6.6	0.84	0.70	0.84	35.8
า	344	3.0	0.432	37.3	LOS D	6.0	42.9	0.87	0.74	0.87	37.3
les	4033	3.0	0.818	29.6	LOS C	30.0	215.1	0.84	0.80	0.87	40.4
	2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	9 Knysna St 2 48 51 1098 82 143 1 1289 aibosch Road 2 155 51 378 82 335 6 Knysna St 2 307 51 1136 82 88 1 1532 otea Park 2 57 51 264 82 23 344	9 Knysna St 2	9 Knysna St 22	9 Knysna St 22						

Intersection of Saasveld Road & Meyer Road

2019 Base Year AM Peak Hour

Move	ment	Performa	nce -	Vehic	les							
Mov ID	Turn	Demand I Total	Flows HV	Deg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed
		veh/h	%	v/c	sec		veh	m				km/h
East:	Saasv	eld										
5	T1	28	3.0	0.052	0.0	LOS A	0.0	0.2	0.02	0.13	0.02	58.8
6	R2	7	3.0	0.052	5.7	LOS A	0.0	0.2	0.02	0.13	0.02	56.5
Appro	ach	36	3.0	0.052	1.2	NA	0.0	0.2	0.02	0.13	0.02	58.3
North:	Meye	r										
7	L2	24	3.0	0.149	8.4	LOS A	0.0	0.0	0.00	1.00	0.00	51.7
9	R2	5	3.0	0.149	8.0	LOS A	0.0	0.0	0.00	1.00	0.00	51.2
Appro	ach	29	3.0	0.149	8.3	LOS A	0.0	0.0	0.00	1.00	0.00	51.6
West:	Saasv	/eld										
10	L2	3	3.0	0.032	5.6	LOS A	0.0	0.0	0.00	0.03	0.00	57.9
11	T1	58	3.0	0.032	0.0	LOS A	0.0	0.0	0.00	0.03	0.00	59.7
Appro	ach	61	3.0	0.032	0.3	NA	0.0	0.0	0.00	0.03	0.00	59.6
All Ve	hicles	126	3.0	0.149	2.4	NA	0.0	0.2	0.00	0.29	0.00	57.2

Move	ment	Performa	ince -	Vehic	les							
Mov ID	Turn	Demand Total	Flows HV	Deg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed
		veh/h	%	v/c	sec		veh	m				km/h
East:	Saasv	eld										
5	T1	51	3.0	0.134	0.0	LOS A	0.1	1.0	0.05	0.18	0.05	58.3
6	R2	21	3.0	0.134	5.7	LOS A	0.1	1.0	0.05	0.18	0.05	56.0
Appro	ach	72	3.0	0.134	1.7	NA	0.1	1.0	0.05	0.18	0.05	57.6
North:	Meye	r										
7	L2	21	3.0	0.134	8.3	LOS A	0.0	0.0	0.00	1.00	0.00	51.7
9	R2	5	3.0	0.134	8.0	LOS A	0.0	0.0	0.00	1.00	0.00	51.2
Appro	ach	26	3.0	0.134	8.2	LOS A	0.0	0.0	0.00	1.00	0.00	51.6
West:	Saasv	/eld										
10	L2	13	3.0	0.028	5.6	LOS A	0.0	0.0	0.00	0.14	0.00	57.0
11	T1	41	3.0	0.028	0.0	LOS A	0.0	0.0	0.00	0.14	0.00	58.7
Appro	ach	54	3.0	0.028	1.3	NA	0.0	0.0	0.00	0.14	0.00	58.3
All Ve	hicles	152	3.0	0.134	2.7	NA	0.1	1.0	0.02	0.31	0.02	56.7

2024 Design Year + Phase 1 Development

AM Peak Hour

Move	ment	Performa	nce -	Vehic	les							
Mov	Turn	Demand I	Flows	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective	Aver. No.	Average
ID	Tuiti	Total	HV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Cycles	Speed
		veh/h	%	v/c	sec		veh	m				km/h
East:	Saasv	eld										
5	T1	87	3.0	0.163	0.0	LOS A	0.1	0.6	0.05	0.15	0.05	58.5
6	R2	27	3.0	0.163	6.5	LOS A	0.1	0.6	0.05	0.15	0.05	56.2
Appro	ach	115	3.0	0.163	1.5	NA	0.1	0.6	0.05	0.15	0.05	57.9
North:	Meye	r										
7	L2	48	3.0	0.333	8.7	LOS A	0.0	0.1	0.00	1.00	0.00	51.8
9	R2	25	3.0	0.333	8.8	LOS A	0.0	0.1	0.00	1.00	0.00	51.3
Appro	ach	74	3.0	0.333	8.8	LOS A	0.0	0.1	0.00	1.00	0.00	51.6
West:	Saasv	/eld										
10	L2	63	3.0	0.128	5.6	LOS A	0.0	0.0	0.00	0.16	0.00	56.9
11	T1	178	3.0	0.128	0.0	LOS A	0.0	0.0	0.00	0.16	0.00	58.6
Appro	ach	241	3.0	0.128	1.5	NA	0.0	0.0	0.00	0.16	0.00	58.1
All Ve	hicles	429	3.0	0.333	2.7	NA	0.1	0.6	0.01	0.30	0.01	56.8

Move	ment	Performa	nce -	Vehic	les							
Mov ID	Turn	Demand I Total	Flows HV	Deg. Satn		Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed
		veh/h	%	v/c	sec		veh	m				km/h
East:	Saasv	eld										
5	T1	318	3.0	0.635	0.1	LOS A	1.4	9.9	0.23	0.18	0.25	57.6
6	R2	111	3.0	0.635	9.6	LOS A	1.4	9.9	0.23	0.18	0.25	55.4
Appro	ach	428	3.0	0.635	2.5	NA	1.4	9.9	0.23	0.18	0.25	57.0
North:	Meye	r										
7	L2	42	3.0	0.658	8.9	LOS A	0.2	1.3	0.00	1.00	0.00	51.9
9	R2	95	3.0	0.658	9.1	LOS A	0.2	1.3	0.00	1.00	0.00	51.4
Appro	ach	137	3.0	0.658	9.0	LOS A	0.2	1.3	0.00	1.00	0.00	51.5
West:	Saasv	eld .										
10	L2	80	3.0	0.135	5.6	LOS A	0.0	0.0	0.00	0.19	0.00	56.6
11	T1	175	3.0	0.135	0.0	LOS A	0.0	0.0	0.00	0.19	0.00	58.3
Appro	ach	255	3.0	0.135	1.8	NA	0.0	0.0	0.00	0.19	0.00	57.8
All Ve	hicles	820	3.0	0.658	3.4	NA	1.4	9.9	0.12	0.32	0.13	56.2

2029 Planning Year + Phase 1+2 Development

AM Peak Hour

Move	ment	Performa	nce -	Vehic	les							
Mov ID	Turn	Demand Total	Flows HV	Deg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed
		veh/h	%	v/c	sec		veh	m				km/h
East:	Saasv	eld										
5	T1	142	3.0	0.256	0.0	LOS A	0.1	0.5	0.04	0.15	0.05	58.2
6	R2	45	3.0	0.256	7.9	LOS A	0.1	0.5	0.04	0.15	0.05	55.9
Appro	ach	187	3.0	0.256	1.9	NA	0.1	0.5	0.04	0.15	0.05	57.6
North:	Meye	r										
7	L2	142	3.0	0.817	26.2	LOS D	0.4	2.6	1.00	1.10	1.44	39.4
9	R2	43	3.0	0.817	52.0	LOS F	0.4	2.6	1.00	1.10	1.44	39.1
Appro	ach	185	3.0	0.817	32.2	LOS D	0.4	2.6	1.00	1.10	1.44	39.3
West:	Saasv	/eld										
10	L2	121	3.0	0.220	5.6	LOS A	0.0	0.0	0.00	0.17	0.00	56.7
11	T1	294	3.0	0.220	0.0	LOS A	0.0	0.0	0.00	0.17	0.00	58.4
Appro	ach	415	3.0	0.220	1.7	NA	0.0	0.0	0.00	0.17	0.00	57.9
All Vel	hicles	787	3.0	0.817	8.9	NA	0.4	2.6	0.24	0.39	0.35	52.0

Move	ment	Performa	nce -	Vehic	les							
Mov ID	Turn	Demand I Total	Flows HV	Deg. Satn		Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed
		veh/h	%	v/c	sec		veh	m				km/h
East:	Saasv	eld										
5	T1	554	3.0	1.046	42.5	LOS E	28.5	204.5	1.00	0.46	4.71	30.3
6	R2	188	3.0	1.046	106.8	LOS F	28.5	204.5	1.00	0.46	4.71	29.6
Appro	ach	742	3.0	1.046	58.9	NA	28.5	204.5	1.00	0.46	4.71	30.1
North:	Meye	r										
7	L2	42	3.0	1.564	521.9	LOS F	51.7	371.4	1.00	4.72	20.06	5.8
9	R2	173	3.0	1.564	557.8	LOS F	51.7	371.4	1.00	4.72	20.06	5.8
Appro	ach	215	3.0	1.564	550.8	LOS F	51.7	371.4	1.00	4.72	20.06	5.8
West:	Saasv	/eld										
10	L2	136	3.0	0.225	5.6	LOS A	0.0	0.0	0.00	0.19	0.00	56.6
11	T1	287	3.0	0.225	0.0	LOS A	0.0	0.0	0.00	0.19	0.00	58.2
Appro	ach	423	3.0	0.225	1.8	NA	0.0	0.0	0.00	0.19	0.00	57.7
All Ve	hicles	1380	3.0	1.564	117.9	NA	51.7	371.4	0.69	1.04	5.65	19.9

2029 Planning Year + Phase 1+2 Development + Upgrades

AM Peak Hour

Move	ment	Performa	nce -	Vehic	les							
Mov	Turn	Demand I	Flows	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective	Aver. No.	Average
ID	Tuiti	Total	HV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Cycles	Speed
		veh/h	%	v/c	sec		veh	m				km/h
East:	Saasv	eld										
5	T1	142	3.0	0.146	4.9	LOS A	1.0	7.0	0.21	0.51	0.21	53.6
6	R2	45	3.0	0.146	8.2	LOS A	1.0	7.0	0.21	0.51	0.21	53.2
Appro	ach	187	3.0	0.146	5.7	LOS A	1.0	7.0	0.21	0.51	0.21	53.5
North:	Meye	r										
7	L2	142	3.0	0.206	7.1	LOS A	1.3	9.0	0.55	0.67	0.55	51.7
9	R2	43	3.0	0.206	10.3	LOS B	1.3	9.0	0.55	0.67	0.55	52.1
Appro	ach	185	3.0	0.206	7.8	LOS A	1.3	9.0	0.55	0.67	0.55	51.7
West:	Saasv	/eld										
10	L2	121	3.0	0.308	5.0	LOS A	2.2	16.0	0.23	0.48	0.23	53.3
11	T1	294	3.0	0.308	5.0	LOS A	2.2	16.0	0.23	0.48	0.23	54.1
Appro	ach	415	3.0	0.308	5.0	LOS A	2.2	16.0	0.23	0.48	0.23	53.9
All Ve	hicles	787	3.0	0.308	5.8	LOS A	2.2	16.0	0.30	0.53	0.30	53.3

Move	ment	Performa	nce -	Vehic	les							
Mov	Turn	Demand	Flows	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective	Aver. No.	Average
ID	Tuiti	Total	HV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Cycles	Speed
		veh/h	%	v/c	sec		veh	m				km/h
East: 3	Saasve	eld										
5	T1	554	3.0	0.680	7.0	LOS A	7.8	55.8	0.77	0.65	0.77	51.8
6	R2	188	3.0	0.680	10.3	LOS B	7.8	55.8	0.77	0.65	0.77	51.4
Appro	ach	742	3.0	0.680	7.8	LOS A	7.8	55.8	0.77	0.65	0.77	51.7
North:	Meye	r										
7	L2	42	3.0	0.245	7.1	LOS A	1.6	11.7	0.60	0.71	0.60	50.4
9	R2	173	3.0	0.245	10.3	LOS B	1.6	11.7	0.60	0.71	0.60	50.8
Appro	ach	215	3.0	0.245	9.7	LOS A	1.6	11.7	0.60	0.71	0.60	50.7
West:	Saasv	reld										
10	L2	136	3.0	0.416	6.5	LOS A	3.4	24.7	0.60	0.61	0.60	52.1
11	T1	287	3.0	0.416	6.5	LOS A	3.4	24.7	0.60	0.61	0.60	52.8
Appro	ach	423	3.0	0.416	6.5	LOS A	3.4	24.7	0.60	0.61	0.60	52.6
All Vel	hicles	1380	3.0	0.680	7.7	LOS A	7.8	55.8	0.69	0.65	0.69	51.8

Access 1 & Meyer Road

2029 Planning Year + Phase 1+2 Development AM Peak Hour

Move	ement	Performa	ince - V	/ehic	les							
Mov	Turn	Demand F	lows D	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective	Aver. No.	Average
ID	Tulli	Total	HV S	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Cycles	Speed
		veh/h	%	v/c	sec		veh	m				km/h
South:	Meye	er										
1	L2	5	3.0 0.	111	5.2	LOS A	0.6	4.0	0.25	0.62	0.25	51.2
2	T1	5	3.0 0.	111	5.2	LOS A	0.6	4.0	0.25	0.62	0.25	52.0
3	R2	118	3.0 0.	111	8.4	LOS A	0.6	4.0	0.25	0.62	0.25	51.6
Appro	ach	128	3.0 0.	111	8.1	LOS A	0.6	4.0	0.25	0.62	0.25	51.6
East:	Access	s 1										
4	L2	38	3.0 0.0	093	4.9	LOS A	0.5	3.4	0.15	0.60	0.15	52.1
5	T1	5	3.0 0.0	093	4.8	LOS A	0.5	3.4	0.15	0.60	0.15	52.9
6	R2	76	3.0 0.0	093	8.1	LOS A	0.5	3.4	0.15	0.60	0.15	52.6
Appro	ach	119	3.0 0.	093	6.9	LOS A	0.5	3.4	0.15	0.60	0.15	52.4
North:	Meye	r										
7	L2	236	3.0 0.2	230	5.6	LOS A	1.3	9.3	0.35	0.56	0.35	52.9
8	T1	15	3.0 0.2	230	5.6	LOS A	1.3	9.3	0.35	0.56	0.35	53.7
9	R2	5	3.0 0.2	230	8.8	LOS A	1.3	9.3	0.35	0.56	0.35	53.3
Appro	ach	256	3.0 0.	230	5.7	LOS A	1.3	9.3	0.35	0.56	0.35	52.9
West:	Arthur	Bleksley										
10	L2	5	3.0 0.0	025	5.8	LOS A	0.1	0.8	0.36	0.59	0.36	51.7
11	T1	5	3.0 0.0	025	5.7	LOS A	0.1	0.8	0.36	0.59	0.36	52.5
12	R2	15	3.0 0.0	025	9.0	LOS A	0.1	0.8	0.36	0.59	0.36	52.1
Appro	ach	25	3.0 0.	025	7.6	LOS A	0.1	0.8	0.36	0.59	0.36	52.1
All Vel	hicles	528	3.0 0.	230	6.7	LOS A	1.3	9.3	0.28	0.59	0.28	52.5

Move	ment P	erformanc	e - Ve	hicles								
Mov	Turn	Demand I	Flows	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective	Aver. No.	Average
ID	Turn	Total	HV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Cycles	Speed
		veh/h	%	v/c	sec		veh	m				km/h
South	: Meyer											
1	L2	17	3.0	0.174	7.0	LOS A	0.9	6.6	0.52	0.71	0.52	50.5
2	T1	17	3.0	0.174	6.9	LOS A	0.9	6.6	0.52	0.71	0.52	51.3
3	R2	123	3.0	0.174	10.1	LOS B	0.9	6.6	0.52	0.71	0.52	50.9
Appro	ach	157	3.0	0.174	9.5	LOS A	0.9	6.6	0.52	0.71	0.52	50.9
East:	Access	1										
4	L2	167	3.0	0.356	4.9	LOS A	2.4	17.4	0.17	0.59	0.17	52.0
5	T1	5	3.0	0.356	4.8	LOS A	2.4	17.4	0.17	0.59	0.17	52.8
6	R2	336	3.0	0.356	8.1	LOS A	2.4	17.4	0.17	0.59	0.17	52.4
Appro	ach	508	3.0	0.356	7.0	LOS A	2.4	17.4	0.17	0.59	0.17	52.3
North:	Meyer											
7	L2	246	3.0	0.240	5.7	LOS A	1.4	10.4	0.38	0.57	0.38	52.8
8	T1	14	3.0	0.240	5.6	LOS A	1.4	10.4	0.38	0.57	0.38	53.7
9	R2	5	3.0	0.240	8.8	LOS A	1.4	10.4	0.38	0.57	0.38	53.3
Appro	ach	265	3.0	0.240	5.7	LOS A	1.4	10.4	0.38	0.57	0.38	52.9
West:	Arthur I	Bleksley										
10	L2	5	3.0	0.028	7.5	LOS A	0.1	1.0	0.55	0.66	0.55	50.7
11	T1	5	3.0	0.028	7.4	LOS A	0.1	1.0	0.55	0.66	0.55	51.5
12	R2	13	3.0	0.028	10.7	LOS B	0.1	1.0	0.55	0.66	0.55	51.1
Appro	ach	23	3.0	0.028	9.2	LOS A	0.1	1.0	0.55	0.66	0.55	51.1
All Ve	hicles	954	3.0	0.356	7.1	LOS A	2.4	17.4	0.30	0.61	0.30	52.2

Access 2 & Saasveld Road

2029 Planning Year + Phase 1+2 Development AM Peak Hour

Move	Movement Performance - Vehicles											
Mov ID	Turn	Demand I Total	Flows HV	Deg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed
		veh/h	%	v/c	sec		veh	m				km/h
East: S	Saasv	eld										
5	T1	38	3.0	0.131	5.1	LOS A	0.7	5.0	0.24	0.60	0.24	52.4
6	R2	118	3.0	0.131	8.3	LOS A	0.7	5.0	0.24	0.60	0.24	52.0
Approa	ach	156	3.0	0.131	7.5	LOS A	0.7	5.0	0.24	0.60	0.24	52.1
North:	Acces	ss 2										
7	L2	38	3.0	0.104	5.4	LOS A	0.5	3.9	0.31	0.61	0.31	51.6
9	R2	76	3.0	0.104	8.6	LOS A	0.5	3.9	0.31	0.61	0.31	52.1
Approa	ach	114	3.0	0.104	7.5	LOS A	0.5	3.9	0.31	0.61	0.31	51.9
West:	Saasv	eld										
10	L2	236	3.0	0.302	5.5	LOS A	1.9	13.4	0.35	0.54	0.35	52.9
11	T1	118	3.0	0.302	5.5	LOS A	1.9	13.4	0.35	0.54	0.35	53.7
Approa	ach	354	3.0	0.302	5.5	LOS A	1.9	13.4	0.35	0.54	0.35	53.2
All Vel	nicles	623	3.0	0.302	6.4	LOS A	1.9	13.4	0.31	0.57	0.31	52.7

Move	ment	Performa	nce -	Vehic	les							
Mov ID	Turn	Demand Total	Flows HV	Deg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed
		veh/h	%	v/c	sec		veh	m				km/h
East:	Saasv	eld										
5	T1	167	3.0	0.326	7.2	LOS A	2.0	14.6	0.60	0.72	0.60	51.8
6	R2	123	3.0	0.326	10.4	LOS B	2.0	14.6	0.60	0.72	0.60	51.5
Appro	ach	291	3.0	0.326	8.6	LOS A	2.0	14.6	0.60	0.72	0.60	51.7
North:	Acces	ss 2										
7	L2	167	3.0	0.424	5.7	LOS A	3.1	22.3	0.42	0.62	0.42	51.3
9	R2	336	3.0	0.424	8.8	LOS A	3.1	22.3	0.42	0.62	0.42	51.8
Appro	ach	503	3.0	0.424	7.8	LOS A	3.1	22.3	0.42	0.62	0.42	51.6
West:	Saasv	/eld										
10	L2	120	3.0	0.217	5.5	LOS A	1.3	9.6	0.36	0.53	0.36	52.8
11	T1	123	3.0	0.217	5.4	LOS A	1.3	9.6	0.36	0.53	0.36	53.7
Appro	ach	243	3.0	0.217	5.5	LOS A	1.3	9.6	0.36	0.53	0.36	53.3
All Ve	hicles	1037	3.0	0.424	7.5	LOS A	3.1	22.3	0.45	0.63	0.45	52.0

Access 3 & Saasveld Road / Kraaibosch Road

2029 Planning Year + Phase 1+2 Development AM Peak Hour

Move	ment F	erformanc	e - Ve	hicles								
Mov	Turn	Demand F	lows	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective	Aver. No.	Average
ID	Turn	Total	HV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Cycles	Speed
		veh/h	%	v/c	sec		veh	m				km/h
South:	Kraaib	osch										
1	L2	118	3.0	0.269	5.0	LOS A	1.6	11.2	0.20	0.49	0.20	53.3
2	T1	236	3.0	0.269	4.9	LOS A	1.6	11.2	0.20	0.49	0.20	54.2
3	R2	5	3.0	0.269	8.2	LOS A	1.6	11.2	0.20	0.49	0.20	53.8
Approa	ach	359	3.0	0.269	5.0	LOS A	1.6	11.2	0.20	0.49	0.20	53.9
East: S	Saasve	ld										
4	L2	5	3.0	0.015	5.5	LOS A	0.1	0.5	0.31	0.55	0.31	52.3
5	T1	5	3.0	0.015	5.5	LOS A	0.1	0.5	0.31	0.55	0.31	53.1
6	R2	5	3.0	0.015	8.7	LOS A	0.1	0.5	0.31	0.55	0.31	52.8
Approa	ach	16	3.0	0.015	6.6	LOS A	0.1	0.5	0.31	0.55	0.31	52.7
North:	Access	s 3										
7	L2	5	3.0	0.112	4.8	LOS A	0.6	4.3	0.09	0.52	0.09	53.2
8	T1	114	3.0	0.112	4.7	LOS A	0.6	4.3	0.09	0.52	0.09	54.0
9	R2	38	3.0	0.112	8.0	LOS A	0.6	4.3	0.09	0.52	0.09	53.6
Approa	ach	157	3.0	0.112	5.5	LOS A	0.6	4.3	0.09	0.52	0.09	53.9
West:	Saasve	eld										
10	L2	118	3.0	0.131	6.2	LOS A	0.7	4.8	0.43	0.61	0.43	52.6
11	T1	5	3.0	0.131	6.2	LOS A	0.7	4.8	0.43	0.61	0.43	53.5
12	R2	5	3.0	0.131	9.4	LOS A	0.7	4.8	0.43	0.61	0.43	53.1
Approa	ach	128	3.0	0.131	6.4	LOS A	0.7	4.8	0.43	0.61	0.43	52.7
All Vel	nicles	660	3.0	0.269	5.4	LOS A	1.6	11.2	0.22	0.52	0.22	53.6

	ement i	Performanc					0.001.00					
Mov	Turn	Demand F		Deg.	Average		95% Back		Prop.		Aver. No.	
ID		Total	HV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Cycles	Speed
		veh/h	%	v/c	sec		veh	m				km/h
South	: Kraait	oosch										
1	L2	123	3.0	0.345	6.0	LOS A	2.1	15.3	0.44	0.58	0.44	52.5
2	T1	246	3.0	0.345	6.0	LOS A	2.1	15.3	0.44	0.58	0.44	53.4
3	R2	5	3.0	0.345	9.2	LOS A	2.1	15.3	0.44	0.58	0.44	53.0
Appro	ach	375	3.0	0.345	6.0	LOS A	2.1	15.3	0.44	0.58	0.44	53.1
East:	Saasve	eld										
4	L2	5	3.0	0.023	9.1	LOS A	0.1	0.8	0.64	0.68	0.64	50.1
5	T1	5	3.0	0.023	9.0	LOS A	0.1	0.8	0.64	0.68	0.64	50.8
6	R2	5	3.0	0.023	12.2	LOS B	0.1	0.8	0.64	0.68	0.64	50.5
Appro	ach	16	3.0	0.023	10.1	LOS B	0.1	0.8	0.64	0.68	0.64	50.4
North	: Acces	s 3										
7	L2	5	3.0	0.441	4.8	LOS A	3.5	25.2	0.13	0.52	0.13	53.0
8	T1	503		0.441	4.8	LOS A	3.5	25.2	0.13	0.52	0.13	53.9
9	R2	167		0.441	8.0	LOS A	3.5	25.2	0.13	0.52	0.13	53.5
Appro	ach	676	3.0	0.441	5.6	LOS A	3.5	25.2	0.13	0.52	0.13	53.8
West:	Saasve	eld										
10	L2	60	3.0	0.074	6.2	LOS A	0.4	2.7	0.45	0.60	0.45	52.5
11	T1	5	3.0	0.074	6.1	LOS A	0.4	2.7	0.45	0.60	0.45	53.3
12	R2	5		0.074	9.4	LOS A	0.4	2.7	0.45	0.60	0.45	53.0
Appro		71		0.074	6.4	LOS A	0.4	2.7	0.45	0.60	0.45	52.6
All Ve	hicles	1137	3 0	0.441	5.8	LOS A	3.5	25.2	0.26	0.54	0.26	53.4
All VE	1110162	1137	3.0	0.441	5.0	LOS A	3.5	25.2	0.20	0.54	0.20	55.4